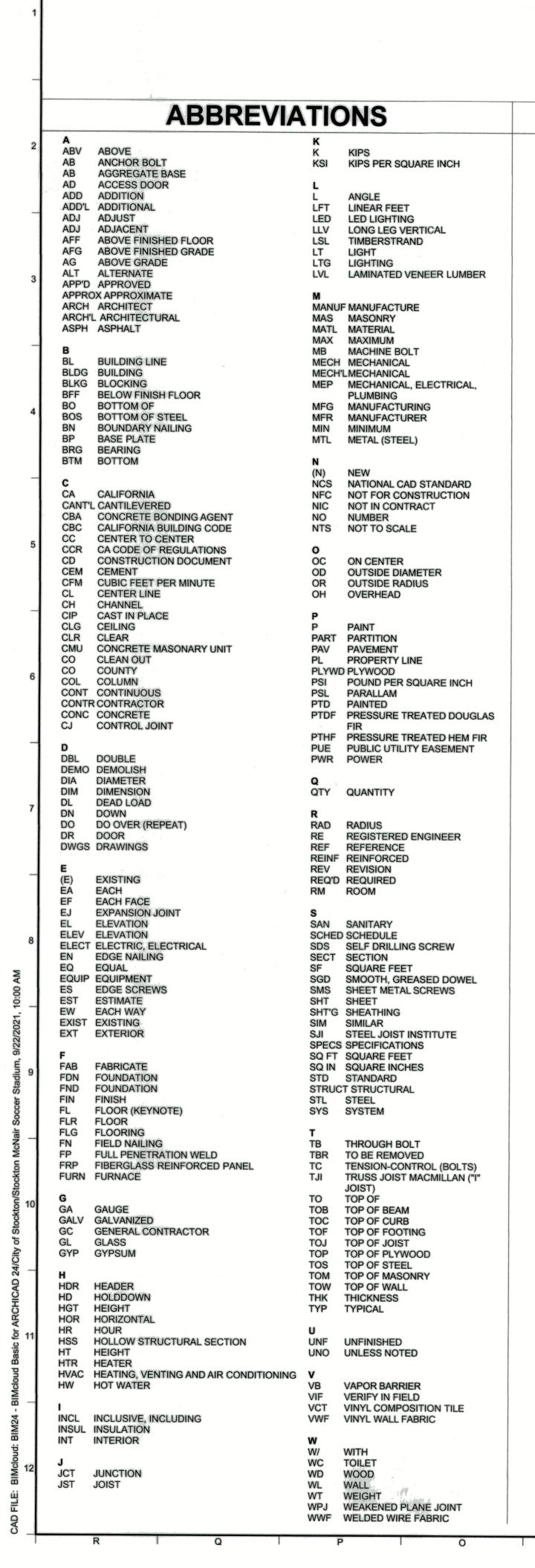
# McNair Soccer Complex Phase - 2

CITY OF STOCKTON CR16024

9820 Ronald E. McNair Way Stockton CA 95210





**INDICATOR** 

**CLIENT** 

CITY OF STOCKTON

Public Works Department

Stockton, CA 95202-2317

ph: (209) 937-7390

ARCHITECTS, LLP

**ARCHITECT** 

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Davis, CA 95616

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Jon Hammond, Architect of Record

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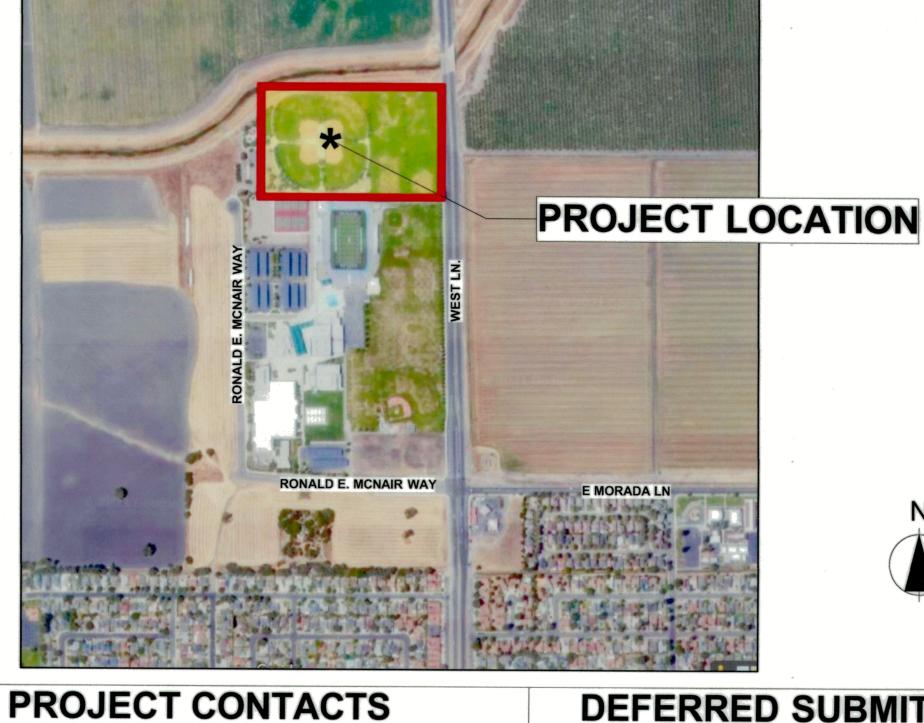
STRUCTURAL ENGINEER

SIEGFRIED ENGINEERING

Sacramento, CA 95825

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IVAN REYNOSO



Sacramento, CA 95825

email: mneils@mneilsengineering.com

ph: 916-923-4400

## **DEFERRED SUBMITTAL**

ELECTRICAL/DATA/TELECOM SCOREBOARDS (TYP. OF 4) FURNISHED BY CITY AND INSTALLED BY CONTRACTOR. M. NEILS ENGINEERING, INC. Stewart Lindsay

1. CONTRACTOR TO PROVIDE POSTS, FOUNDATIONS, ELECTRICAL & DATA CONNECTIONS, AND ALL REQUIRED TRENCHING, PATCHING & REPAIRING PER CONSTRUCTION DOCUMENTS.

CONTRACTOR TO PROVIDE DEFERRED ENGINEERING AND SHOP DRAWINGS FOR ANY INSTALLATION REQUIREMENTS NOT ALREADY ADDRESSED IN THESE CONSTRUCTION DOCUMENTS.

## SPECIAL INSPECTION

SEE S1.1 FOR STATEMENT OF SPECIAL INSPECTION REQUIREMENTS.

## **APPLICABLE CODES**

CONSTRUCTION WILL COMPLY WITH ALL APPLICABLE CODES AND ORDINANCES AS AMENDED BY THE STATE OF CALIFORNIA AND LOCAL JURISDICTION.

2019 California Building Code (CBC) 2019 California Electrical Code, California Code of Regulations Title 24, Part 3 2019 California Fire Code 2019 California Energy Code San Joaquin County Codes and Ordinances

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Consultant

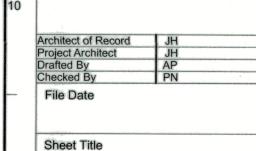
City Approvals

Issue: 100% CDs W/ PLAN CHECK CORRECTIONS 9-20-2021

No.	Date	Description
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McNair Soccer Complex Phase - 2

9820 Ronald E. McNair Way Stockton CA 95210



**COVER SHEET** 

Project Number CR16024

Reference **Sheet Scale** 

**Sheet Number** G0.1

5463C

#### **GENERAL NOTES**

- ALL IMPROVEMENTS SHALL BE CONSTRUCTED IN STRICT ACCORDANCE WITH THE FOLLOWING: CURENT CITY OF STOCKTON STANDARD SPECIFICATIONS AND PLANS. INCLUSIVE OF ALL CURENT REVISIONS AND AMENDMENTS. CALIFORNIA DEPARTMENT OF TRANSPORTATION CURRENT STANDARD PLANS AND SPECIFICATIONS (CALTRANS), INCLUSIVE OF ALL CURRENT REVISIONS AND AMENDMENTS, AND CA-MUTCD LATEST EDITION, INCLUSIVE OF ALL CURRENT REVISIONS AND AMENDMENTS THERETO. WHERE THERE IS A CONFLICT BETWEEN THE PLANS AND THE CITY'S STANDARD SPECIFICATIONS AND PLANS, THE CITY OF STOCKTON STANDARD SPECIFICATIONS AND PLANS SHALL PREVAIL. CONTRACTOR SHALL BE RESPONSIBLE FOR CONSTRUCTING THE IMPROVEMENTS IN ACCORDANCE WITH THE ABOVE-MENTIONED STANDARDS AND SPECIFICATIONS.
- 2. THE CONTRACTOR SHALL VISIT THE SITE AND BECOME FAMILIAR WITH THE COMPLETE WORK SCOPE AND ALL RELATED CONDITIONS PRIOR TO BID. ANY QUESTIONS OR DISCREPANCIES WITH THE INFORMATION SHOWN HEREIN MUST BE DIRECTED TO THE ENGINEER PRIOR TO BID.
- 3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL PERMITS AND LICENSES REQUIRED FOR THE CONSTRUCTIONS AND COMPLETION OF THE PROJECT AND SHALL PERFORM ALL WORK IN ACCORDANCE WITH THE REQUIREMENTS AND CONDITIONS OF ALL PERMITS AND APPROVALS APPLICABLE TO THIS PROJECT. THE CONTRACTOR SHALL ENSURE THAT THE NECESSARY PERMITS AND/OR LICENSES ARE SECURED PRIOR TO CONSTRUCTION.
- 4. CONTRACTOR SHALL OBTAIN AN ENCROACHMENT PERMIT FROM THE CITY OF STOCKTON FOR ANY WORK DONE WITHIN CITY RIGHTS-OF-WAY OR ON CITY-OWNED FACILITIES WITHIN AN EASEMENT. CONTRACTOR SHALL CALL THE PERMIT CENTER AT (209) 937 - 8366 TO REQUEST A CONTROL NUMBER AND ACTIVATE THE PERMIT NO LESS THAN 24 HOURS. BUT NOT IN EXCESS OF 72 HOURS PRIOR TO START OF WORK.
- THE CONTRACTOR SHALL RECEIVE PRIOR APPROVAL FROM THE ENGINEER FOR ANY EXTRA WORK. THE CONTRACTOR SHALL BE HELD RESPONSIBLE FOR ANY FIELD CHANGES MADE WITHOUT PRIOR WRITTEN AUTHORIZATION FROM THE ENGINEER AT NO ADDITIONAL COST TO THE CITY.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING FROM DAMAGE ALL EXISTING AND NEWLY PLACED IMPROVEMENTS THAT ARE TO REMAIN. SUCH IMPROVEMENTS THAT ARE DAMAGED BY THE CONTRACTOR SHALL BE REPLACED AT NO ADDITIONAL COST TO THE CITY.
- 8. THE CONTRACTOR AGREES THAT HE SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT INCLUDING SAFETY AND SECURITY OF JOB SITE. THAT THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS.
- THE CONTRACTOR SHALL MAINTAIN A NEATLY MARKED SET OF FULL-SIZE AS-BUILT DRAWINGS SHOWING THE FINAL LOCATION OF FINAL IMPROVEMENTS. AS-BUILT DRAWINGS SHALL REFLECT CHANGE ORDERS, ACCOMMODATIONS, AND ADJUSTMENTS TO ALL IMPROVEMENTS CONSTRUCTED, WHERE NECESSARY, SUPPLEMENTAL DRAWINGS SHALL BE PREPARED AND SUBMITTED BY THE CONTRACTOR.
- 10. PRIOR TO ACCEPTANCE OF THE PROJECT. THE CONTRACTOR SHALL DELIVER TO THE ENGINEER, ONE SET OF NEATLY MARKED AS-BUILT DRAWINGS. AS-BUILT DRAWINGS SHALL BE REVIEWED AND THE COMPLETE AS-BUILT DRAWING SET SHALL BE CURRENT WITH ALL CHANGES AND DEVIATIONS REDLINED AS A PRECONDITION TO THE FINAL PROGRESS PAYMENT APPROVAL AND/OR FINAL ACCEPTANCE.
- 11. ALL TRENCH EXCAVATION SHALL BE IN ACCORDANCE WITH SECTION 7 OF THE CITY OF STOCKTON STANDARD SPECIFICATIONS.
- 12. THE CONTRACTOR SHALL PROVIDE ALL SHORING, BRACING, SLOPING OR OTHER PROVISIONS NECESSARY TO PROTECT WORKERS FOR ALL AREAS TO BE EXCAVATED TO A DEPTH OF 5' OR MORE. EXCAVATIONS OF 5 FEET OR MORE IN DEPTH WILL REQUIRE AN EXCAVATIONS PERMIT FROM THE STATE OF CALIFORNIA DEPARTMENT OF INDUSTRIAL SAFETY FOR TRENCHES S FEET OR MORE IN DEPTH. THE CONTRACTOR SHALL COMPLY WITH SECTION 7-1.02K(6)(b) OF THE CALTRANS STANDARDS, SECTION 6705 OF THE STATE OF CALIFORNIA LABOR CODE, AND ANY LOCAL CODES OR ORDINANCES.

- 13. ATTENTION IS CALLED TO: SECTION 1541(b)(I) OF THE CONSTRUCTION SAFETY ORDERS (CALIFORNIA CODE OF REGULATIONS, TITLE 8), ISSUED BY THE OCCUPATIONAL SAFETY AND HEALTH STANDARDS BOARD PURSUANT TO THE CALIFORNIA OCCUPATIONAL SAFETY AND HEALTH ACT OF 1973. WHICH STATES: "THE APPROXIMATE LOCATION OF SUBSURFACE INSTALLATIONS, SUCH AS SEWER, TELEPHONE, FUEL, ELECTRIC, WATER LINES, OR ANY OTHER SUBSURFACE INSTALLATIONS THAT REASONABLY MAY BE EXPECTED TO BE ENCOUNTERED DURING EXCAVATION WORK, SHALL BE DETERMINED BY THE EXCAVATOR PRIOR TO OPENING AN EXCAVATION."
- 14. PRIOR TO COMMENCING ANY WORK, IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO HAVE EACH UTILITY COMPANY LOCATE IN THE FIELD THEIR MAIN AND SERVICE LINES. THE CONTRACTOR SHALL NOTIFY MEMBERS OF THE UNDERGROUND SERVICE ALERT (U.S.A.) 48 HOURS IN ADVANCE OF PERFORMING EXCAVATION WORK BY CALLING THE TOLL-FREE NUMBER (800) 227-2600.
- 15. IT SHALL BE CONTRACTOR'S SOLE RESPONSIBILITY TO PROTECT ALL EXISTING UTILITIES SO THAT NO DAMAGE RESULTS TO THEM DURING THE PERFORMANCE OF HIS CONTRACT. THE CONTRACTOR SHALL BE REQUIRED TO COOPERATE WITH OTHER CONTRACTORS AND UTILITY COMPANIES INSTALLING NEW OR MODIFIED STRUCTURES, UTILITIES AND SERVICES WITHIN THE PROJECT LIMITS.
- 16. THE CONTRACTOR SHALL EXERCISE DUE CAUTION AND SHALL CAREFULLY PRESERVE BENCHMARKS, CONTROL POINTS, REFERENCE POINTS AND ALL SURVEY MONUMENTS, AND SHALL BEAR ALL EXPENSES FOR REPLACEMENT AND/OR ERROR CAUSED BY HIS UNNECESSARY LOSS OR DISTURBANCE. THE CONTRACTOR SHALL CONSULT WITH A LICENSED LAND SURVEYOR OR CIVIL ENGINEER LICENSED TO PRACTICE LAND SURVEYING IN CALIFORNIA PRIOR TO BEGINNING CONSTRUCTION TO ENSURE THAT ANY PRECONSTRUCTION CORNER RECORDS, AS REQUIRED BY THE STATE OF CALIFORNIA PROFESSIONAL LAND SURVEYOR ACT HAVE BEEN FILED WITH THE COUNTY SURVEYOR, PURSUANT TO SECTION 8771(a-f) OF THE CALIFORNIA BUSINESS AND PROFESSION CODE.
- 17. ALL WORK IN THE PUBLIC RIGHT-OF-WAY IS SUBJECT TO THE APPROVAL AND ACCEPTANCE OF THE ENGINEER.
- 18. PRIOR TO PLACEMENT OF ANY FINISH ASPHALT CONCRETE OR CONCRETE. THE CONTRACTOR SHALL VERIFY ALL FINISH GRADES AND SLOPES FOR COMPLIANCE WITH THE AMERICANS WITH DISABILITIES ACT (ADA) AND OBTAIN APPROVAL AND ACCEPTANCE BY THE ENGINEER.
- 19. THE CONTRACTOR SHALL LAYOUT IMPROVEMENTS FROM THE DIMENSIONS SHOWN ON THE PLANS. ANY CLARIFICATION OR CONFLICTS. DISCREPANCIES OR AMBIGUITIES SHALL BE DIRECTED TO THE ENGINEER PRIOR TO THE CONSTRUCTION OF THE IMPROVEMENTS.
- 20. DUST CONTROL SHALL BE PERFORMED AT ALL TIMES, AT THE CONTRACTORS' EXPENSE, TO MINIMIZE ANY DUST NUISANCE AND SHALL BE IN ACCORDANCE WITH SECTION 10-5 OF CALTRANS STANDARD SPECIFICATIONS AND THE REQUIREMENTS OF THE CITY OF STOCKTON.
- 21. THE CONTRACTOR SHALL MAINTAIN ALL EXISTING WATER, SEWER, AND DRAINAGE FACILITIES WITHIN THE CONSTRUCTION AREA UNTIL NEW IMPROVEMENTS ARE IN PLACE AND FUNCTIONING, EXCEPT WHERE OTHERWISE APPROVED.
- 22. INGRESS AND EGRESS BY PROPERTY OWNERS, BUSINESSES, AND OTHERS SHALL BE PROVIDED AT ALL TIMES DURING THE COURSE OF CONSTRUCTION UNLESS OTHERWISE APPROVED OR SPECIFIED.
- 23. SIDEWALK REMOVAL SHALL BE TO THE NEAREST SCORE MARK OR AS DETERMINED BY THE ENGINEER. CONTRACTOR SHALL NEATLY SAW-CUT CONCRETE WHERE PULL BOXES ARE TO BE PLACED AND SHALL RESTORE THE SLAB TO MATCH THE EXISTING CONDITION.
- 24. NEW SIDEWALK SHALL BE DOWELED INTO EXISTING SIDEWALK ACCORDING TO CITY STANDARD DRAWING NO. R-55.

#### TRAFFIC SIGNAL AND ELECTRICAL NOTES:

INSTALLATION OF NEW CONDUCTORS INTO EXISTING CONDUIT SHALL BE IN ACCORDANCE WITH SECTION 77-1.12 OF THE SPECIAL PROVISIONS. PRIOR TO INSTALLATION OF NEW CONDUCTORS/CABLES IN EXISTING CONDUITS, THE CONTRACTOR SHALL USE CABLE LOOSENER TO LOOSEN THE CONDUITS. THE CONTRACTOR SHALL ALSO USE PULLING LUBRICANT FOR PULLING WIRES, AND A PULL TAPE CONFORMING TO THE PROVISION DESCRIBED UNDER "CONDUIT". ELSEWHERE IN THE SPECIAL PROVISIONS.

- POLES, PULL BOXES, DETECTOR HANDHOLES, INDUCTIVE LOOPS AND CONTROLLER CABINET LOCATIONS SHALL BE LOCATED IN THE FIELD BY THE CONTRACTOR WITH THE APPROVAL OF THE CITY TRAFFIC ENGINEER. TYPICALLY, DETECTOR HANDHOLES SHOULD BE INSTALLED ON LANE LINES.
- CONTRACTOR SHALL MEET GENERAL ORDER (G.O.) 95 REQUIREMENTS AND LOCATE FOUNDATIONS SO AS TO PROVIDE A MINIMUM OF 6' RADIAL CLEARANCE FROM ALL EQUIPMENT TO OVERHEAD POWER LINES (PRIMARY) AND A MINIMUM OF 3' RADIAL CLEARANCE TO COMMON NEUTRAL LINES. SIGNAL POLES SHALL BE LOCATED TO PROVIDE A MINIMUM OF 10' RADIAL CLEARANCE TO PRIMARY LINES. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY COSTS ASSOCIATED WITH WORKING WITHIN THE 10' RADIAL CLEARANCE ZONE.
- CONDUIT ROUTING SHOWN IS DIAGRAMMATICALLY. CONTRACTOR SHALL LAYOUT RUNS TO SUIT FIELD CONDITIONS AND THE COORDINATION REQUREMENTS OF UTILITIES OR ANY OTHER TRADES, AND TO THE SATISFACTION OF THE CITY OF STOCKTON. UPON COMPLETION OF CONDUIT INSTALLATION, THE ACTUAL LOCATION OF THE CONDUITS SHALL BE NOTED ON AN AS-BUILT SET OF PRINTS AND FURNISHED TO THE CITY.

#### TRAFFIC STAGING NOTES:

- THE CONTRACTOR SHALL MAINTAIN ALL TRAFFIC CONTROL DEVICES AT ALL TIMES.
- 2. ALL TRAFFIC CONTROL DEVICES SHALL BE REMOVED FROM VIEW WHEN NOT IN USE.
- THE ENGINEER HAS THE AUTHORITY TO INITIATE FIELD CHANGES AS NECESSARY IN THE INTEREST OF PUBLIC SAFETY.
- ROAD CLOSURES SHALL REQUIRE WRITTEN APPROVAL FROM THE ENGINEER.
- ALL NIGHT WORK WILL REQUIRE WRITTEN APPROVAL FROM THE ENGINEER. LANE CLOSURES, ROAD DETOURS, ROAD CLOSURES, AND TRAFFIC SIGNAL MODIFICATIONS ASSOCIATED WITH OVERNIGHT CONSTRUCTION ACTIVITIES WILL REQUIRE WARNING SIGNS BE PLACED AT LEAST ONE WEEK IN ADVANCE OF STARTING CONSTRUCTION
- CONTRACTOR SHALL PROVIDE ADEQUATE TEMPORARY LIGHTING DURING THE COURSE OF ALL NIGHT WORK.
- 7. ALL WORKERS SHALL BE EQUIPPED WITH AN ORANGE SAFETY VEST (OR REFLECTIVE VEST AT NIGHT).
- TRENCHES MUST BE BACKFILLED OR PLATED DURING NON-WORKING HOURS.
- REFER TO SECTION 12 OF THE SPECIAL PROVISIONS REGARDING TEMPORARY ACCESS ROUTES FOR PEDESTRIANS (INCLUDING ADA) AND BICYCLISTS.
- 10. TEMPORARY "NO PARKING" SIGNS SHALL BE POSTED THREE (3) WORKING DAYS PRIOR TO COMMENCING WORK.
- 11. ACCESS TO DRIVEWAYS SHALL BE MAINTAINED AT ALL TIMES UNLESS OTHER ARRANGEMENTS ARE MADE. SIGNS ON ROADWAY SHALL NOT BLOCK DRIVEWAY.
- 12. TRAFFIC CONTROL PLANS SHOWN HEREON ARE FOR GUIDANCE ONLY. CONTRACTOR SHALL PREPARE TRAFFIC CONTROL PLANS IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS. TO BE SUBMITTED FOR REVIEW AND APPROVAL PRIOR TO COMMENCING WORK.

ARCHITECTS. LLI 909 FIFTH STREET, DAVIS, CA 530.750.0756 WWW.INDIGOARCH.COM City Approvals Issue: 100% CDs W/ PLAN CHECK CORRECTIONS 9-20-2021 Description McNair Soccer Complex Phase - 2 9820 Ronald E. McNair Way Stockton CA 95210

Architect of Record	JH	
Project Architect	JH	
Drafted By	AP	
Checked By	PN	
File Date		

**GENERAL NOTES** 

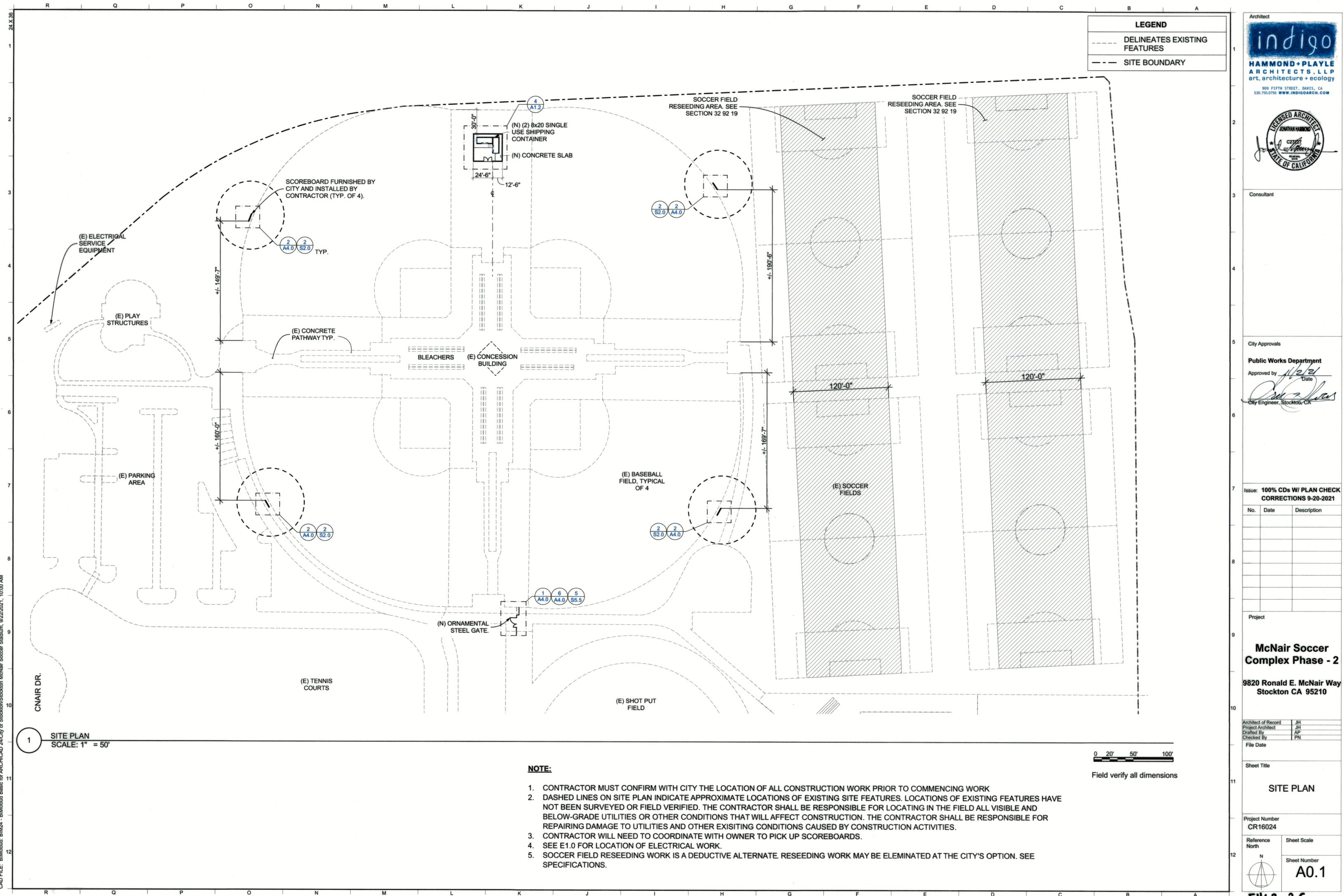
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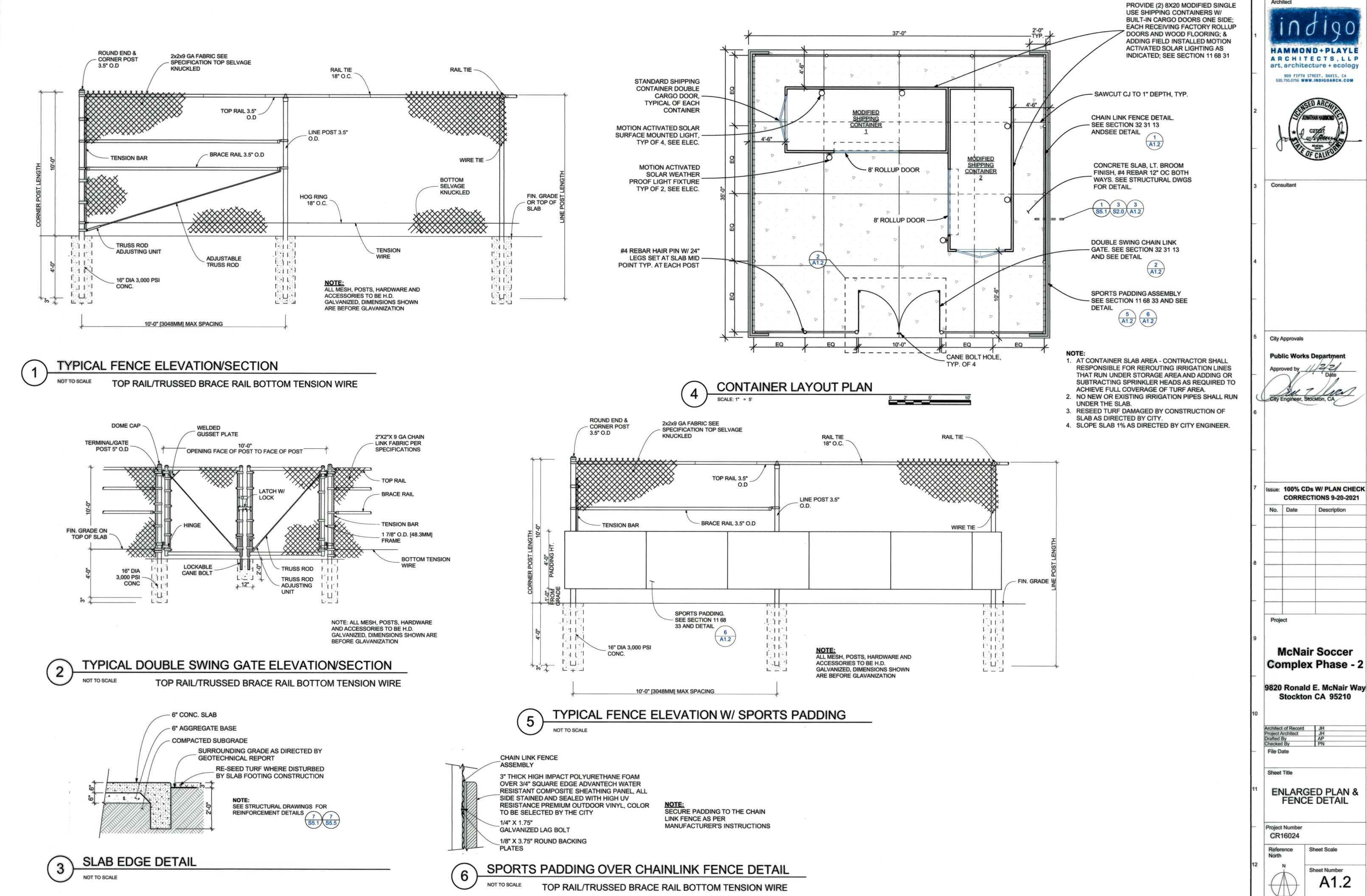
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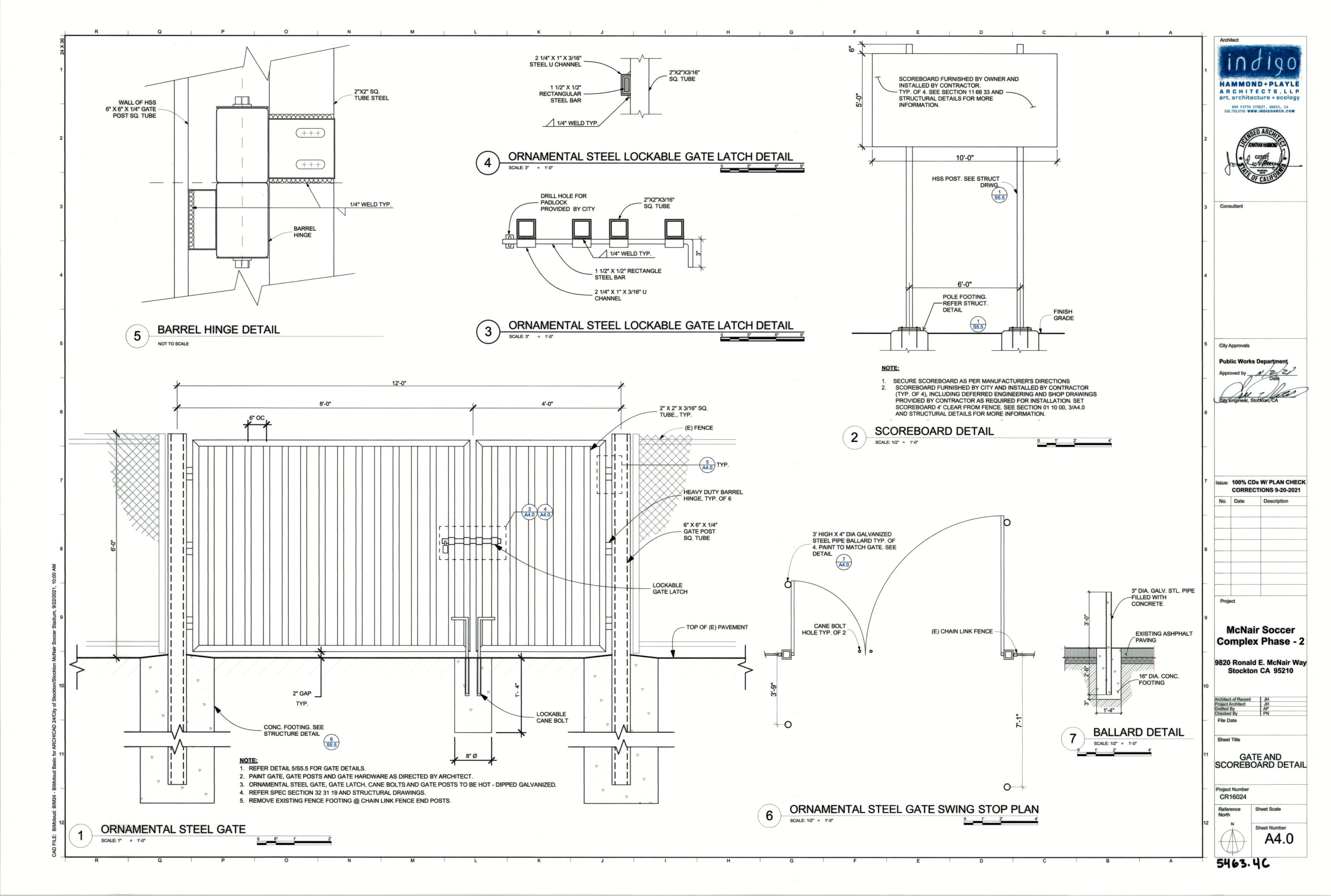
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- B. NOTES AND DETAILS ON THE STRUCTURAL DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN OR NOTED IN ANY PART OF THE WORK THE
- C. DETAILS SHOWN ON THE STRUCTURAL DRAWINGS SHALL APPLY IN ESTIMATING AND CONSTRUCTION TO EVERY LIKE CONDITION WHETHER OR NOT THE REFERENCE IS REPEATED.

DETAILS FOR OTHER SIMILAR WORK SHALL APPLY.

- D. THE STRUCTURAL DRAWINGS AND DETAILS SHALL NOT BE SCALED. CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO STARTING CONSTRUCTION. COORDINATE ALL PLAN DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
- E. DIMENSIONS SHOWN SHALL TAKE PRECEDENCE OVER SCALE ON PLANS AND DETAILS, ANY DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE STRUCTURAL ENGINEER.
- F. SITE CONDITIONS THAT ARE NOT REFLECTED ON THE STRUCTURAL DRAWINGS OR THAT DEVIATE FROM THE MAXIMUM OR MINIMUM DIMENSIONS INDICATED SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER IN A TIMELY MANNER. SUCH CONDITIONS MAY INCLUDE CONFLICT IN GRADES, ADVERSE SOIL CONDITIONS, GROUND WATER PRESENT, DEEPENED FOOTINGS, UNCOVERED AND UNEXPECTED UTILITY LINES, ETC.
- G. THE REVIEW OF SHOP DRAWINGS BY THE STRUCTURAL ENGINEER IS FOR GENERAL COMPLIANCE WITH THE DRAWINGS AND SPECIFICATIONS.
- H. ALL DETAIL CHANGES DESIRED SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER IN WRITING. SHOP DRAWINGS DO NOT CONSTITUTE CHANGES IN WRITING. THE APPROVING GOVERNMENT AGENCY MAY REQUIRE APPROVAL OF THE FIELD CHANGE PROPOSED PRIOR TO CONSTRUCTION.
- THE RESPONSIBILITY FOR REVIEW AND COORDINATION OF DRAWINGS AND SPECIFICATIONS PRIOR TO THE START OF RELATED CONSTRUCTION SHALL BEAR ON THE CONTRACTOR. DISCREPANCIES THAT EXIST SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER IN A TIMELY MANNER, PRIOR TO START OF RELATED CONSTRUCTION.
- J. LOADS TO THE BUILDING THAT EXCEED THE LOADS INDICATED IN THE STRUCTURAL DRAWINGS, OR ANY LOADS EXCEEDING 400 POUNDS THAT ARE NOT INDICATED ON THE STRUCTURAL DRAWINGS SHALL BE REPORTED TO THE STRUCTURAL ENGINEER.
- K. CONSTRUCTION/BUILDING MATERIALS SHALL BE SPREAD OUT IF PLACED ON THE STRUCTURE, THE MATERIAL LOADS SHALL NOT EXCEED THE DESIGN LIVE LOAD INDICATED. WHERE THE STRUCTURE HAS NOT ATTAINED FINAL DESIGN STRENGTH OR SUFFICIENT STABILITY ADEQUATE SHORING AND OR BRACING SHALL BE INSTALLED TO TEMPORARILY SUPPORT THE MATERIALS.
- L. THE STRUCTURAL DRAWINGS SHOW THE REQUIREMENTS FOR THE COMPLETED STRUCTURE ONLY. TEMPORARY WORKS REQUIRED TO COMPLETE THE CONSTRUCTION PROCESS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- M. DEVIATIONS FROM EXISTING CONDITIONS WHERE INDICATED ON THE STRUCTURAL DRAWINGS SHALL BE RESOLVED WITH THE STRUCTURAL ENGINEER PRIOR TO PROCEEDING WITH DEMOLITION WORK OR NEW CONSTRUCTION.
- N. THE RESPONSIBILITY FOR NECESSARY SHORING OR BRACING OF THE STRUCTURE DURING CONSTRUCTION OR DEMOLITION SHALL BEAR ON THE CONTRACTOR. THESE STRUCTURAL DRAWINGS DO NOT INDICATE THE PHASING OF WORK. THE RESPONSIBILITY FOR SCHEDULING AND COORDINATION OF THE WORK SHALL BEAR ON THE CONTRACTOR.
- O. CONTRACTOR SHALL INVESTIGATE THE SITE DURING CLEARING AND EARTHWORK OPERATIONS FOR FILLED EXCAVATIONS OR BURIED STRUCTURES SUCH AS CESSPOOLS, CISTERNS, FOUNDATIONS, UTILITIES, ETC. IF ANY SUCH STRUCTURES ARE FOUND, THE STRUCTURAL ENGINEER SHALL BE NOTIFIED IMMEDIATELY.
- P. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING SITE IMPROVEMENTS. ANY IMPROVEMENTS DAMAGED BY THE CONTRACTOR DURING THE COURSE OF WORK SHALL BE REPORTED TO THE CITY REPRESENTATIVE IMMEDIATELY AND SHALL BE REPAIRED TO THE SATISFACTION OF THE CITY REPRESENTATIVE AT THE CONTRACTORS EXPENSE. PRIOR TO FINAL ACCEPTANCE, THE CITY PARKS SECTION SHALL BE AFFORDED THE OPPORTUNITY TO TEST EXISTING IMPROVEMENTS FOR FUNCTIONALITY. THE CONTRACTOR SHALL ADVISE THE CITY PROJECT MANAGER TWO WEEKS AHEAD OF WHEN THE SITE WILL BE READY FOR SUCH TESTING.
- Q. INTERRUPTION OF MAINTENANCE OPERATIONS: BETWEEN THE 1ST OF MAY AND OCTOBER 31ST., THE PARK IRRIGATION SYSTEM SHALL NOT BE TURNED OFF FOR ANY LONGER THAN 3 CONSECUTIVE DAYS WITHIN ANY 10-DAY PERIOD, AT THE DISCRETION OF THE CITY PARKS MANAGER. IN NO CASE SHALL THE CONTRACTOR'S OPERATIONS CAUSE DISRUPTION OF MAINTENANCE OPERATIONS WITHOUT TWO WEEKS PRIOR COORDINATION WITH THE CITY PARKS SECTION THROUGH THE CITY'S PROJECT MANAGER.

#### 2. DESIGN DATA:

A. WIND DESIGN DATA:

BASIC DESIGN WIND SPEED (V-LRFD): 95 MPH RISK CATEGORY: WIND EXPOSURE: DESIGN WIND PRESSURE (FREE STANDING WALLS AND SIGNS - LRFD): 28.9 PSF (AT 12'-6" AFF) **SCOREBOARDS** 

B. EARTHQUAKE DESIGN DATA:

RISK CATEGORY: SEISMIC IMPORTANCE FACTOR 1.0 MAPPED SPECTRAL RESPONSE: SS = 0.681 S1 = 0.273 SITE CLASS:

DESIGN SPECTRAL RESPONSE: SDS = 0.570 SD1 = 0.374 SEISMIC DESIGN CATEGORY: LONGITUDE/LATITUDE:

38.04292/-121.298122

3. POST INSTALLED ANCHORS:

A. GENERAL

- 1) ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S INSTALLATION INSTRUCTIONS AND THE REQUIREMENTS OF THE APPLICABLE CODE REPORT. SPECIAL INSPECTION FOR ALL POST INSTALLED ANCHORS AS NOTED IN THE CODE REPORT.
- ANCHORS SHALL BE OF THE TYPE, DIAMETER AND MINIMUM DIMENSIONAL REQUIREMENTS (EMBEDMENT, SPACING, EDGE DISTANCE) AS INDICATED ON THE DRAWINGS. ALTERNATE PRODUCTS SHALL CARRY AN EVALUATION APPROVAL AND SHALL BE APPROVED BY THE STRUCTURAL ENGINEER IN WRITING PRIOR TO DELIVERY TO THE JOBSITE.
- 3) ANCHORS SHALL BE INSTALLED IN HOLES DRILLED WITH DRILLING EQUIPMENT OF THE TYPE REQUIRED IN THE MANUFACTURERS PUBLISHED CODE REPORT. HOLES SHALL BE CLEANED IN CONFORMANCE WITH THE ANCHOR MANUFACTURERS INSTALLATION INSTRUCTIONS.
- 4) ANCHOR SHALL BE TIGHTENED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.

WHEN INSTALLING ANCHORS OR DRILL AND EPOXY REBAR INTO EXISTING REINFORCED CONCRETE AVOID CUTTING OR DAMAGING THE EXISTING REINFORCING BARS OR REINFORCING TENDONS. LOCATE THE REINFORCING USING A NON-DESTRUCTIVE METHOD PRIOR TO INSTALLATION, DAMAGED REBAR MAY REQUIRE REPAIR CONTACT STRUCTURAL ENGINEER WHEN REBAR IS DAMAGED OR CUT.

- B. ADHESIVE ANCHORS OR REBAR INSTALLED WITH ADHESIVE:
- 1) ADHESIVE ANCHOR INSTALLERS SHALL BE TRAINED BY A QUALIFIED REPRESENTATIVE OF ADHESIVE MANUFACTURER ON THE PROPER PROCEDURES AND TECHNIQUES FOR INSTALLATION.
- 2) ADHESIVE SHALL BE STORED ON THE JOBSITE IN A COOL, DRY LOCATION IN CONFORMANCE WITH THE MANUFACTURER'S REQUIREMENTS.
- 3) ADHESIVE ANCHORS SHALL NOT BE USED FOR OVERHEAD INSTALLATION UNLESS SPECIFICALLY DETAILED IN THE STRUCTURAL DRAWINGS. PISTON PLUGS OR SIMILAR SHALL BE USED DURING OVERHEAD INSTALLATION
- 4) ADHESIVE ANCHORS SHALL BE INSTALLED WHILE CONCRETE IS DRY AND ALLOWED TO CURE FULLY PRIOR TO REINTRODUCING WATER TO SYSTEM.
- 5) ADHESIVE ANCHORS FOR INSTALLATION IN SOLID NORMAL-WEIGHT CONCRETE SHALL BE ONE OF THE FOLLOWING AS NOTED ON THE PLAN:

BRAND/TYPE CODE REPORT HILTI HIT-RE 500 V3 (ESR-3814) HILTI HIT-HY 200 (ESR-3187) SIMPSON SET-3G (ESR-4057) SIMPSON AT-XP (ER-263) SIMPSON SET-XP (ESR-2508)

6) ADHESIVE ANCHORS FOR INSTALLATION IN MASONRY SHALL BE ONE OF THE FOLLOWING AS NOTED ON THE PLAN:

BRAND/TYPE CODE REPORT HILTI HIT-HY 270 (ESR-4143) HOLLOW MASONRY UNIT HILTI HIT-HY 270 (ESR-4144) UN-REINFORCED MASONRY HILTI HIT-HY-200 (ESR-3963) SOLID GROUTED MASONRY SIMPSON SET-XP (ER-265) SOLID GROUTED MASONRY SIMPSON AT-XP (ER-281) SOLID GROUTED MASONRY

C. EXPANSION ANCHORS FOR INSTALLATION IN CONCRETE OR MASONRY SHALL BE ONE OF THE FOLLOWING AS NOTED ON THE PLAN:

HILTI KWIK BOLT (TZ) (ESR-1917) CONCRETE HILTI KWIK BOLT (TZ) (ESR-3785) SOLID GROUTED MASONRY SIMPSON STRONG-BOLT 2 (ESR-3037) CONCRETE SIMPSON STRONG-BOLT 2 (ESR-240) SOLID GROUTED MASONRY SIMPSON WEDGE-ALL (ESR-1396) SOLID GROUTED MASONRY

D. SCREW ANCHORS IN CONCRETE OR MASONRY SHALL BE ONE OF THE FOLLOWING AS NOTED ON THE PLAN:

BRAND/TYPE CODE REPORT

HILTI KH-EZ (ESR-3027) CONCRETE HILTI KH-EZ (ESR-3056) SOLID GROUTED MASONRY SIMPSON TITEN HD (ESR-2713) CONCRETE

SIMPSON TITEN HD (ESR 1056) SOLID GROUTED MASONRY E. POWDER ACTIVATED FASTENERS (PAF) IN CONCRETE, MASONRY OR STRUCTURAL STEEL SHALL BE ONE OF

THE FOLLOWING AS NOTED ON THE PLAN:

CODE REPORT HILTI X-U (ESR-2269) CONCRETE/CMU/STEEL HILTI X-P (ESR-2269) CONCRETE SIMPSON PDPA-XXX (ESR-2138) CONCRETE/MASONRY SIMPSON PDPA-XXK (ESR-2138) STEEL

#### 4. FOUNDATION:

A. FOUNDATION DESIGN CRITERIA:

1) THE FOUNDATION SYSTEM WAS PREPARED IN ACCORDANCE WITH THE RECOMMENDATIONS MADE BY THE GEOTECHNICAL ENGINEER OF RECORD. FOR MORE INFORMATION REFER TO THE FOLLOWING GEOTECHNICAL INVESTIGATION REPORT:

**BSK ASSOCIATES** 3140 GOLD CAMP DRIVE #160 RANCHO CORDOVA, CA 95670 620-108-115

2) ALLOWABLE SOIL BEARING PRESSURE SHALL BE 2,000 PSF (D+L) ON PROPERLY COMPACTED FILL SOIL. THE ALLOWABLE BEARING NOTED ABOVE MAY BE INCREASED BY 1/3 RD FOR SHORT TERM LOADING (SEISMIC AND WIND LOAD CASES).

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B. FOUNDATION DESIGN CRITERIA:

11/23/2020

2) DESIGN COEFF. OF SLIDING FRICTION

1) MIN FOOTING EMBED. BELOW ADJ. FIN. GRADE: 24 INCHES

DESIGN PASSIVE PRESSURE: 250 PCF

4) DESIGN ACTIVE PRESSURE: 60 PCF

- B. FOUNDATION CONSTRUCTION:
- 1) SPECIAL INSPECTION AND TESTING IS REQUIRED IN ACCORDANCE WITH THE BUILDING CODE, SEE "STATEMENT OF SPECIAL INSPECTIONS" ON THESE CONSTRUCTION DOCUMENTS. EXCAVATIONS FOR FOOTINGS SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE.
- 2) PLACE REINFORCING STEEL AND CONCRETE AS SOON AS POSSIBLE AFTER EXCAVATION FOR THE FOUNDATION, USE NECESSARY SPACERS AND DOBIE/CHAIR/STANDS AS REQUIRED.
- 3) CONCRETE SHALL OBTAIN THE SPECIFIED 28-DAY CONCRETE COMPRESSIVE STRENGTH PRIOR TO BACK-FILLING UNLESS OTHERWISE APPROVED BY THE STRUCTURAL ENGINEER.

THE APPROVAL OF THE GEOTECHNICAL ENGINEER. FLOODING WILL NOT BE PERMITTED.

5) ALL ABANDONED FOOTINGS, UTILITIES, ETC, THAT INTERFERE WITH THE NEW CONSTRUCTION SHALL BE

4) FOOTING AND UTILITY TRENCH BACKFILL SHALL BE MECHANICALLY COMPACTED IN LAYERS SUBJECT TO

6) FOOTING DEPTHS INDICATED ON THE STRUCTURAL DRAWINGS ARE MINIMUMS ASSUMING THAT SUITABLE BEARING MATERIALS AND SOIL PREPARATION HAS BEEN MET PER THE GEOTECHNICAL REPORT. THE GEOTECHNICAL INSPECTOR MAY REQUIRE INCREASED FOUNDATION DEPTHS TO MEET THE ASSUMED IN-SITU CONDITIONS. THE RESPONSIBILITY FOR CONFORMING TO THE GEOTECHNICAL REPORT RECOMMENDATIONS SHALL BEAR ON THE CONTRACTOR.

- 7) CAST-IN-PLACE ANCHOR BOLTS, DOWELS AND HOLD-DOWN ANCHORS SHALL BE TIED IN PLACE PRIOR TO FOUNDATION INSPECTION.
- 8) TEMPORARY SHORING SHALL BE PROVIDED FOR RETAINING WALLS THAT ARE DESIGNED AS "RESTRAINED" AT BOTH THE TOP AND BOTTOM OF THE WALL. SHORING SHALL REMAIN IN PLACE UNTIL CONCRETE REACHES THE 28 DAY DESIGN STRENGTH PRIOR TO BACKFILLING OF THE WALLS.

#### 5. REINFORCED CONCRETE:

- ALL CONCRETE WORK SHALL BE PERFORMED IN ACCORDANCE WITH ACI 318 AND CBC BUILDING CODES.
- 2) SPECIAL INSPECTION AND TESTING IS REQUIRED IN ACCORDANCE WITH THE BUILDING CODE, SEE "STATEMENT OF SPECIAL INSPECTIONS" ON THESE CONSTRUCTION DOCUMENTS.
- 3) READY MIX CONCRETE SHALL BE MIXED AND DELIVERED IN ACCORDANCE WITH ASTM C94. CONCRETE SHALL NOT BE PLACED BEYOND 1 1/2 HOURS FOLLOWING BATCHING
- 4) CEMENT SHALL CONFORM TO ASTM C150 TYPE I OR II, LOW ALKALI.
- 5) AGGREGATES FOR NORMAL WEIGHT CONCRETE SHALL CONFORM TO ASTM C33. NORMAL WEIGHT CONCRETE SHALL HAVE A MAXIMUM DRY DENSITY OF 150 PCF.
- 6) AGGREGATES FOR LIGHTWEIGHT CONCRETE SHALL BE EXPANDED SHALE TYPE AND CONFORM TO ASTM C330. LIGHTWEIGHT CONCRETE SHALL HAVE A MAXIMUM DRY DENSITY OF 115 PCF.
- 7) FLY ASH SHALL CONFORM TO ASTM C618 CLASS F. FLY ASH SHALL BE LIMITED TO NO MORE THAN 15% OF THE TOTAL WEIGHT OF CEMENTITIOUS MATERIALS IN THE CONCRETE DESIGN WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER.
- 8) THE CONTRACTOR SHOULD BE AWARE THAT FLY ASH USE MAY SIGNIFICANTLY RETARD THE SETTING TIME OF CONCRETE POTENTIALLY DELAYING FINISHING OPERATIONS. FURTHER FLY ASH IS EXPECTED TO RETARD THE EARLY CONCRETE STRENGTH (BEFORE 28 DAYS) THUS IF HIGH EARLY STRENGTH IS REQUIRED BY THE CONSTRUCTION SCHEDULE THE AMOUNT OF FLY ASH SHOULD BE REDUCED AND/OR THE MIX DESIGN AND/OR CURING METHODS MUST BE USED TO COMPENSATE AS NECESSARY.
- 9) CONCRETE MIX SHALL BE DESIGNED BY QUALIFIED TESTING LABORATORY AND SHALL BEAR THE SEAL AND SIGNATURE OF A CALIFORNIA LICENSED CIVIL ENGINEER. THE MIX DESIGN SHALL STATE THE PROJECT NAME AND INTENDED USAGE OF THE CONCRETE.
- 10)MIX DESIGNS SHALL BE SUBMITTED TO STRUCTURAL ENGINEER OF RECORD (SEOR) FOR REVIEW AND APPROVAL PRIOR TO USE.
- 11) CONSTRUCTION AND CONTROL JOINTS SHALL BE PROVIDED AS INDICATED FOR WALLS, SLABS AND OTHER ELEMENTS. THE LOCATIONS OF JOINTS NOT SPECIFICALLY INDICATED SHALL BE REVIEWED BY THE STRUCTURAL ENGINEER AND APPROVED BY THE ARCHITECT.
- 12) CONDUIT, PIPES OR DUCTS SHALL NOT BE PLACED IN CONCRETE ON METAL DECK, CONCRETE COLUMNS, CONCRETE WALLS OR CONCRETE FOOTINGS UNLESS SPECIFICALLY INDICATED ON THE STRUCTURAL DRAWINGS OR APPROVED IN WRITING BY THE STRUCTURAL ENGINEER.
- 13) WHERE PERMITTED BY STRUCTURAL ENGINEER, CONDUIT OR SLEEVES WITHIN SLABS AND WALLS SHALL BE PLACED WITHIN THE MIDDLE THIRD OF THE THICKNESS OF THE SLAB OR WALL. THE MAXIMUM SIZE OF CONDUIT IS ¼ OF THE SLAB OR WALL THICKNESS. CONDUITS OR SLEEVES SHALL BE SPACED AT LEAST 3 TIMES THEIR DIAMETER ON CENTER. IT IS NOT ACCEPTABLE TO CUT OR REDISTRIBUTE REBAR THAT MAY CONFLICT WITH THE CONDUIT OR SLEEVE.

#### B. CONCRETE STRENGTH

1) CONCRETE SHALL BE HARD ROCK AND OBTAIN A MINIMUM 28 DAY DESIGN COMPRESSIVE STRENGTH (F'C) AS FOLLOWS:

DESCRIPTION	(F'C)	MAX W/C
(a) SPREAD FOOTINGS	3500 PSI	0.45
(b) POLE FOUNDATIONS	3500 PSI	0.45
(c) SLAB ON GRADE	3500 PSI	0.45

#### 2) MINIMUM CEMENT REQUIREMENT:

(a) 5.0 SACK PER YARD FOR 2,500 PSI CONCRETE. (b) 5.5 SACK PER YARD FOR 3,000 PSI CONCRETE. (c) 6.0 SACK PER YARD FOR 3,500 PSI CONCRETE.

3) SLUMP: 4" (+/-) MAX. WITHOUT SUPERPLASTICIZER ADDED.

- 4) CURING: KEEP WET MINIMUM OF 7 DAYS OR APPLY MEMBRANE CURING COMPOUND APPROVED BY
- 5) VIBRATION: ALL CONCRETE SHALL BE VIBRATED INTO PLACE WITH A MECHANICAL VIBRATOR.
- 6) FORM REMOVAL: SIDE FORMS OF FOUNDATIONS AND SLABS AFTER A MINIMUM OF 2 DAYS.
- 7) CONCRETE IN AREAS OF SHEAR WALLS, COLUMNS, BEAMS OR SLABS WHERE REBAR APPEARS CONGESTED SHALL INCLUDE A SUPERPLASTICIZER AND/OR SMALLER AGGREGATE SIZE WITH APPROPRIATE ADJUSTMENTS FOR AIRE AND CEMENT CONTENT AS NECESSARY.
- 8) WHERE SPECIFIED COLUMN CONCRETE STRENGTH EXCEEDS THAT OF THE SURROUNDING ELEVATED SLAB IT IS NECESSARY TO PUDDLE CONCRETE OF THE HIGHER STRENGTH FOR A PERIMETER OF 24" BEYOND THE COLUMN FACE.
- B. REINFORCING MATERIALS:
- 1) REINFORCING BARS SHALL CONFORM TO ASTM A615 GRADE 60, UNO.
- REINFORCING BARS FOR COLUMNS, WALLS AND SEISMIC ELEMENTS SHALL CONFORM TO ASTM A706
- 3) WELDED REINFORCING BARS SHALL BE APPROVED BY STRUCTURAL ENGINEER IN WRITING BEFORE FABRICATION. ONLY ASTM A706 GRADE 60 CAN BE WELDED WHEN APPROVED OR DETAILED.
- 4) WELDED WIRE REINFORCEMENT (WWF) SHALL CONFORM TO ASTM A1064 GRADE 80 USING DEFORMED
- 5) FOR SLABS ON GRADE, REINFORCING SHALL BE HELD IN PLACE AT THE ELEVATION NOTED ON THE DRAWINGS BY MEANS OF CONCRETE BLOCKS, BAR CHAIRS OR OTHER APPROVED METHOD. BARS SHALL

NOT BE POSITIONED BY PULLING UP WITH HOOKS AS CONCRETE IS POURED.

- 6) ALL REINFORCING STEEL SHALL BE DETAILED AND PLACED IN CONFORMANCE WITH BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318) AND THE MANUAL OF STANDARD PRACTICE FOR REINFORCED CONCRETE CONSTRUCTION (CRSI) AS MODIFIED BY THE PROJECT DRAWINGS.
- ALL REINFORCING BAR BENDS SHALL BE MADE COLD.

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- 8) WHERE HOOKS ARE ILLUSTRATED AS 90-DEGREE HOCKS, 180-DEGREE HOOKS MAY BE USED IN LIEU OF
- 9) REINFORCING BAR LAP SPLICES SHALL BE CLASS "B" UNO. LAP SPLICES SHALL NOT BE PERMITTED IN MOMENT FRAMES OR SHEAR WALLS UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS OR APPROVED BY THE ENGINEER IN WRITING.
- 10) MINIMUM LAP OF WELDED WIRE FABRIC SHALL BE 12 INCHES OR ONE FULL MESH AND ONE HALF, WHICHEVER IS GREATER.
- 11) DOWELS BETWEEN FOOTINGS AND WALLS OR COLUMNS SHALL BE THE SAME GRADE, SIZE SPACING AND NUMBER AS THE SPECIFIED VERTICAL REINFORCING, UNO.
- C. ANCHOR BOLTS (UNLESS NOTED OTHERWISE ON PLANS):
- HOOKED BOLTS, HEADED BOLTS, THREADED RODS:
- (a) SILL PLATES AND WOOD LEDGERS: ASTM F1555-36 (b) HOLDDOWN BOLTS: ASTM F1554-36 (NO HOOKS) (c) WALL ANCHORS: ASTM F1554-36 (NO HOOKS)
- (d) HIGH STRENGTH ANCHORS: ASTM F1554-55 (e) PEMB ANCHOR BOLTS: F1554-55.
- 2) ANCHOR BOLTS EXTENDING THROUGH PRESSURE TREATED WOOD SHALL BE NOT DIPPED GALVANIZED AT LEAST FOR THE PORTION OF THE BOLTS IN CONTACT WITH PRESSURE TREATMENT. HOT DIP GALVANIZED IS NOT REQUIRED FOR ANCHOR BOLTS WITH SBX/D.O.T. TREATED WOOD AT MUDSILLS WITHIN WEATHER-TIGHT WALLS.
- ANCHOR BOLTS SHALL BE HAND TIGHT IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS.
- D. MISCELLANEOUS:
- PROVIDE ¾" CHAMFER ON ALL EXPOSED CONCRETE EDGES UNLESS OTHERWISE NOTED ON THE PLANS.
- 2) ANCHOR BOLTS AND DOWELS MUST BE SECURELY SUPPORTED IN PLACE BEFORE POURING CONCRETE TO ENSURE PROPER LOCATION PER PLAN.
- E. VAPOR BARRIER: ASTM E 1745 CLASS A (SAME AS W.R. MEADOWS PERMINATOR, STEGO VAPOR BARRIER, OR RYDEN VAPOR BLOCK (10 MIL MIN)
- F. CONCRETE BONDING AGENT (CBA):
  - SIKA ARMATEC 110 EPOCHEM
  - 2) WR MEADOWS SEALTIGHT INTRALOK OR APPROVED EQUIVALENT.
  - 3) PRIOR TO APPLYING CBA, EXISTING CONCRETE SURFACE SHALL BE ROUGHENED TO A 1/2" AMPLITUDE AND CLEANED OF ALL DEBRIS, APPLY CBA PER MANUFACTURERS RECOMMENDATIONS.
- G. FLATNESS CATEGORY USES:
- CONVENTIONAL = UTILITY SPACES LEFT EXPOSED, NO APPLIED FINISHES.
- 2) MODERATELY FLAT = CARPETED FINISHES IN COMMERCIAL BUILDINGS AND LOW SPEED VEHICULAR
- 3) FLAT = SUITABLE FOR TILE, VINYL TILE, AND CONVENTIONAL FORK LIFT TRAFFIC IN INDUSTRIAL
- 4) VERY FLAT = HIGH-END INDUSTRIAL APPLICATIONS FOR HIGH SPEED FORK LIFTS AND OTHER
- SUPER FLAT = SPECIALTY APPLICATIONS.

PRODUCTION EQUIPMENT.

#### 6. STRUCTURAL STEEL:

- A. STRUCTURAL STEEL WORK SHALL BE PERFORMED IN ACCORDANCE WITH CHAPTER 22 OF CBC AND AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND AISC 303 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES."
- B. MATERIALS:
- STRUCTURAL STEEL:
- (a) W SHAPES: ASTM A992, FY = 50 KSI. SHAPES WITH FLANGES 1 1/2" THICK & THICKER, AND PLATES THAT ARE 1 1/2" THICK OF THICKER IN BUILT-UP CROSS-SECTION SHALL HAVE MINIMUM CHARPY
- V-NOTCH TOUGHNESS OF 20 FT-LBS AT 70 DEGREES F. (b) OTHER ROLLED SHAPES: ASTM A36, FY = 36 KSI.
- (c) RECTANGULAR TUBING (HSS) A500, GRADE B, FY = 46 KSI.
- (d) ROUND TUBING (HSS): ASTM A500 GR. B, FY = 42KSI. (e) PIPE: ASTM A53 GR B, FY = 35 KSI.
- (f) PLATE & BAR: ASTM A36, FY = 36 KSI.
- HIGH STRENGTH BOLTS: ASTM A325 TYPE 1 SNUG TIGHT PER RCSC "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS" UNLESS NOTED OTHERWISE. IT IS ACCEPTABLE TO USE A3125 SPECIFICATION AS SUBSTITUTE WHERE A325 IS NOT AVAILABLE.
- 3) COMMON BOLTS: ASTM A307 GR. A SNUG TIGHT PER RCSC.
- 4) ANCHOR BOLTS: ASTM F1554 GR. 36 UNLESS NOTED OTHERWISE. ANCHOR BOLTS EXTENDING THROUGH PRESSURE TREATED WOOD SHALL BE HOT DIPPED GALVANIZED AT LEAST FOR THE PORTION OF THE BOLTS IN CONTACT WITH PRESSURE TREATMENT. HOT DIP GALVANIZED IS NOT REQUIRED FOR ANCHOR BOLTS WITH D.O.T. TREATED WOOD AT MUDSILL WITHIN WEATHER-TIGHT WALLS.
- 5) NUTS: PLAIN ASTM A563 GR. C HEAVY HEX, GALVANIZED ASTM A563 GR. DH HEAVY HEX.
- 6) WASHERS: PLAIN ASTM F436 ROCKWELL C HARDNESS 38 MIN, DIRECT TENSION INDICATING WASHERS -ASTM F959.

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3428 Brookside Road

City Approvals

Issue: 100% CDs W/ PLAN CHECK CORRECTIONS 9-20-2021

Description

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Project

McNair Soccer Complex Phase - 2

9550 Ronald E. McNair Way Stockton CA 95210

File Date

**GENERAL NOTES** 

Project Number CR 16024

Sheet Title

Reference Sheet Scale Sheet Number

(a) FILLER MATERIAL PER AWS D1.1, MINIMUM CHARPY V-NOTCH TOUGHNESS 20 FT-LBS AT 20 DEGREES F AS FOLLOWS:

- i. SMAW-E7018
- ii. FCAW-E70T-X LH
- iii. GMAW-E70S-X LH iv. SAW-F71-EL12 LH

#### (a) WELD METAL:

- FILLET WELDS AND PARTIAL JOINT PENETRATION GROOVE WELDS IN MATERIAL LESS THAN 1
  ½" THICK. MINIMUM CHARPY V-NOTCH TOUGHNESS: NO REQUIREMENT.
- ii. COMPLETE JOINT PENETRATION GROOVE AND PARTIAL JOINT PENETRATION GROOVE WELDS IN MATERIAL 1 1/2" THICK AND THICKER. MINIMUM CHARPY V-NOTCH TOUGHNESS 20 FT-LBS AT 0 DEGREES F. AND 40 FT-LB AT 70 DEGREES F.
- 8) PAINT: PER ARCHITECTURAL PLANS. PAINT PRIMER SSPC-PAINT 23 TO BE APPLIED OVER STEEL SURFACES WITH A MINIMUM SURFACE CLEANLINESS OF SSPC-SP6.
- 9) GALVANIZING REQUIRED WHERE NOTED ON THE PLANS OR AT STRUCTURAL STEEL PERMANENTLY EXPOSED TO WEATHER UNLESS OWNER AND ENGINEER APPROVE AN ALTERNATE METHOD OF STEEL PROTECTION.
- (a) STRUCTURAL SHAPES, PLATE AND BAR: ASTM A123.
- (b) HARDWARE: ASTM A153. THE PURCHASE OF GALVANIZED NUTS AND BOLTS SHALL BE FROM THE SAME SUPPLIER WITH THE NUTS LUBRICATED AND TESTED WITH THE SUPPLIED BOLTS. NUTS SHALL BE OVER-TAPPED AFTER GALVANIZING IN ACCORDANCE WITH ANSI B 18.2.2 BOLT THREADS SHALL NOT BE RE-CUT AFTER GALVANIZING. NUTS AND BOLTS SHALL BE SHIPPED TOGETHER IN THE SAME SHIPPING CONTAINER. REPAIR OF THE DAMAGED AND UNCOATED AREAS OF HOT-DIP GALVANIZED COATINGS SHALL BE IN ACCORDANCE WITH ASTM A780.

10) STAINLESS STEEL REQUIRED WHERE NOTED ON THE PLANS.

(a) SHAPES AND BAR: ASTM A276 TYPE 304 OR 316 ROCKWELL B HARDNESS 80 MIN.
(b) PLATE: ASTM A240 TYPE 304 OR 316 ROCKWELL BE HARDNESS OF 80 MINIMUM.

- (c) WELDING: PER AWS D1.6, FILLER MATERIAL AS FOLLOWS:i. TYPE 304 E308-XX/ER308-XX.
  - ii. TYPE 316 E316-XX/ER316-XX.

(a) BOLTS AND NUTS: ASTM F593 GROUP 2 ROCKWELL B HARDNESS 85 MINIMUM.

- C. FABRICATION:
- 1) WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS ONLY IN AN AWS APPROVED WELD SHOP.
- 2) SHOP FABRICATION TO THE GREATER POSSIBLE EXTENT.
- SHOP PRIME ALL STEEL EXCEPT GALVANIZED STEEL. DO NOT PRIME SURFACES TO BE WELDED OR WHICH WILL BE PLACED INTO OR COVERED WITH CONCRETE.
- 4) BOLT HOLES TO BE DRILLED MAXIMUM 1/16 INCH LARGER THAN BOLT DIAMETER UNLESS OTHERWISE NOTED ON PLANS.
- D. SHOP DRAWINGS:
- 1) SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER AND ARCHITECT FOR ALL STRUCTURAL STEEL MEMBERS.
- 2) CONTRACTOR TO COORDINATE SHOP DRAWINGS WITH ALL TRADES.
- E. TESTING AND INSPECTION:
- 1) STRUCTURAL STEEL: SUBMIT MILL REPORTS AS PER AISC SPECIFICATIONS.
- 2) HIGH-STRENGTH NUTS BOLTS AN WASHERS: PROVIDE WRITTEN MILL CERTIFICATES.
- 3) ALL FIELD WELDING SHALL BE BY A CERTIFIED WELDER AND INSPECTED BY AN APPROVED WELDING
- 4) HIGH-STRENGTH BOLTS NOTED AS TENSION-CONTROL (TC) ON PLANS SHALL BE INSPECTED BY A QUALIFIED INSPECTOR APPROVED BY THE ENGINEER.
- PRE-ENGINEERED ELEMENTS
- A. PRE-ENGINEERED STRUCTURAL AND NON-STRUCTURAL ELEMENTS AND THEIR ATTACHMENTS, WHICH ARE NOT DETAILED ON THESE DRAWINGS, THAT ARE SUPPORTED BY THE BUILDING OR STRUCTURE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND DESIGNED BY OTHERS. IT IS ASSUMED THESE ITEMS MAY INCLUDE:
- 1) SCOREBOARD & FRAME.
- B. DESIGN CALCULATION AND SHOP/ERECTION DRAWINGS, SIGNED BY A CALIFORNIA REGISTERED CIVIL ENGINEER, SHALL BE SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION AND INSTALLATION. CALCULATIONS AND DRAWINGS SHALL CLEARLY IDENTIFY LOADS IMPOSED ON THE BUILDING STRUCTURE.
- C. IT IS ASSUMED THESE ITEMS ARE DEFERRED APPROVALS TO THE BUILDING OFFICIAL.

#### 8. STATEMENT OF SPECIAL INSPECTION

- A. GENERAL:
- USE OF THESE CONSTRUCTION DRAWINGS REQUIRES COMPLIANCE WITH SPECIAL INSPECTION PROGRAM AS OUTLINED IN THE CONSTRUCTION DOCUMENTS. FURTHER THE CONTRACTOR AND/OR OWNER MUST COMPLY WITH THE WITH ANY AND ALL SPECIAL INSPECTION REQUIREMENTS OF THE BUILDING OFFICIAL.
- SPECIAL INSPECTIONS PER CBC CHAPTER 17 ARE REQUIRED FOR THIS PROJECT, THE FOLLOWING IS A STATEMENT OF SPECIAL INSPECTIONS:
- 8.1) THE OWNER OR OWNER'S AGENT, OTHER THAN THE CONTRACTOR, SHALL EMPLOY SPECIAL INSPECTION AND TESTING AGENCIES TO PROVIDE INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS.
- 8.2) SPECIAL INSPECTION SHALL BE PERFORMED IN ADDITION TO INSPECTION BY THE BUILDING OFFICIAL AS REQUIRED IN SECTION 110 OF THE BUILDING CODE. SPECIAL INSPECTION SHALL NOT BE A SUBSTITUTE FOR INSPECTION BY THE BUILDING OFFICIAL.
- 8.3) WHEN WORK IN MORE THAN ONE CATEGORY OF WORK REQUIRING SPECIAL INSPECTION OR TESTING IS TO BE PERFORMED SIMULTANEOUSLY, OR THE GEOGRAPHIC LOCATION OF THE WORK IS SUCH THAT IT CANNOT BE OBSERVED IN ACCORDANCE WITH THE STATEMENT OF SPECIAL

INSPECTIONS AND SECTION 1704 OF THE BUILDING CODE, IT SHALL BE THE SPECIAL INSPECTION AGENCY'S RESPONSIBILITY TO EMPLOY A SUFFICIENT NUMBER OF INSPECTORS TO ASSURE THAT THE REQUIRED WORK IS INSPECTED.

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- 8.4) THE SPECIAL INSPECTION AGENCY SHALL BE APPROVED BY THE BUILDING OFFICIAL FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION
- 8.5) THE CONSTRUCTION MATERIALS TESTING AGENCY SHALL BE APPROVED BY THE BUILDING OFFICIAL FOR THE TESTING OF MATERIALS. SYSTEMS, COMPONENTS AND EQUIPMENT.
- 8.6) WORK REQUIRING SPECIAL INSPECTION OR TESTING THAT IS INSTALLED OR COVERED WITHOUT THE APPROVAL OF THE BUILDING OFFICIAL IS SUBJECT TO REMOVAL OR EXPOSURE AT THE CONTRACTOR'S EXPENSE.
- 8.7) THE SPECIAL INSPECTION AGENCY SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE.
- 8.8) SPECIAL INSPECTION REPORTS SHALL INDICATE WHETHER THE WORK INSPECTED WAS, OR WAS NOT PERFORMED IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.
- 8.9) THE CONSTRUCTION MATERIALS TESTING AGENCY SHALL FURNISH REPORTS TO THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE.
- 8.10) MATERIAL TESTING REPORTS SHALL INDICATE WHETHER THE TESTED MATERIALS CONFORM, OR DO NOT CONFORM. TO THE REQUIREMENTS OF THE APPROVED CONSTRUCTION DOCUMENTS
- 8.11) DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO COMPLETION OF THAT PHASE OF WORK.
- 8.12) A FINAL REPORT DOCUMENTING THE REQUIRED SPECIAL INSPECTIONS, MATERIAL TESTING AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED AT A POINT IN TIME AGREED UPON, PRIOR TO THE START OF WORK, BY THE PERMIT APPLICANT AND THE BUILDING OFFICIAL.
- 8.13) SPECIAL INSPECTION AND TESTING IS REQUIRED FOR THE OFF-SITE FABRICATION OF STRUCTURAL STEEL AND REINFORCING ASSEMBLIES, UNLESS THE FABRICATOR MEETS THE EXCEPTIONS OF CBC SECTION 1704.2.5.
- 8.14) STRUCTURAL OBSERVATIONS BY THE STRUCTURAL ENGINEER OF RECORD (SEOR) SHALL BE NECESSARY FOR STRUCTURES ASSIGNED TO RISK CATEGORY III OR IV, SDC E OR F, OR FOR ANY BUILDING GREATER THAN 75 FEET FROM FINISHED GRADE. CONTACT SEOR TO SCHEDULE OBSERVATIONS FOR THE FOLLOWING WHERE REQUIRED:
- (a) AFTER REBAR PLACEMENT, BEFORE POURING FOUNDATIONS.
- (b) AFTER REBAR PLACEMENT, BEFORE CLOSING CONCRETE SHEAR WALL FORMS.
- (c) AFTER REBAR PLACEMENT, BEFORE CLOSING CONCRETE MOMENT FRAME FORMS.
- (d) AFTER REBAR PLACEMENT, BEFORE GROUTING MASONRY SHEAR WALLS.
- (e) AFTER ERECTION OF STEEL BRACED FRAMES, BEFORE BRACE IS HIDDEN BY OTHER CONSTRUCTION.(f) AFTER ERECTION OF STEEL MOMENT FRAMES, BEFORE FRAME IS HIDDEN BY OTHER CONSTRUCTION.
- 8.15) ADDITIONAL SUBMITTALS MAY BE REQUIRED BY THE CONTRACTOR TO THE BUILDING OFFICIAL AS OUTLINED IN CBC SECTIONS 1704.4 AND 1704.5 REGARDING COMPLIANCE WITH SPECIAL INSPECTION PROCEDURES.
- 8.16) WHERE PERIODIC SPECIAL INSPECTION IS REQUIRED, THE INSPECTOR SHALL BE INTERMITTENTLY PRESENT WHERE WORK TO BE INSPECTED HAS BEEN OR IS BEING PERFORMED (NOT CONTINUOUS).
- 8.17) WHERE CONTINUOUS SPECIAL INSPECTION IS REQUIRED, FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION, BY INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED.

#### B. SOILS

- SPECIAL INSPECTIONS AND TESTS OF EXISTING SITE SOIL CONDITIONS, FILL PLACEMENT AND LOAD-BEARING REQUIREMENTS SHALL BE IN ACCORDANCE WITH CBC SECTION 1705.6 AND TABLE 1705.6. THE APPROVED GEOTECHNICAL REPORT AND CONSTRUCTION DOCUMENTS PREPARED BY THE REGISTERED DESIGN PROFESSIONAL(S) SHALL BE USED TO DETERMINE COMPLIANCE.
- 2) IT IS ASSUMED THAT GEOTECHNICAL INSPECTIONS SHALL BE PERFORMED BY THE GEOTECHNICAL ENGINEER OF RECORD, OR HIS OR HER QUALIFIED REPRESENTATIVE. CONSTRUCTION OF FOUNDATION, SLAB ON GRADE, PAVEMENT, ETC. SHALL BE IN SUBSTANTIAL CONFORMANCE WITH GEOTECHNICAL REPORT RECOMMENDATIONS.
- 3) A GEOTECHNICAL INSPECTION REPORT, SIGNED, STAMPED AND DATED BY THE GEOTECHNICAL ENGINEER OF RECORD, SHALL BE SUBMITTED TO AND APPROVED BY THE BUILDING OFFICIAL PRIOR TO THE CONTRACTOR REQUESTING A BUILDING DEPARTMENT FOUNDATION INSPECTION OR THE PLACEMENT OF FOUNDATION CONCRETE. THE REPORT SHALL STATE THAT:
  - a. THE BUILDING PAD WAS PREPARED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT.
  - b. THE UTILITY TRENCHES HAVE BEEN PROPERLY BACKFILLED AND COMPACTED.
  - c. THE FOUNDATION EXCAVATIONS COMPLY WITH THE INTENT OF THE GEOTECHNICAL REPORT.

### C. CONCRETE:

- SPECIAL INSPECTIONS AND TEST OF CONCRETE CONSTRUCTION SHALL BE PER CBC SECTION 1705.3 AND TABLE 1705.3.
- 2) SPECIAL INSPECTION SHALL BE PRESENT DURING TAKING OF TEST SPECIMENS AND PLACING OF REINFORCED CONCRETE.
- 3) SPECIAL INSPECTION NEED NOT BE PRESENT CONTINUOUSLY DURING THE PLACING OF REINFORCING STEEL AND PRESTRESSING TENDONS, PROVIDED THE SPECIAL INSPECTOR HAS INSPECTED FOR CONFORMANCE TO THE APPROVED PLANS PRIOR TO THE CLOSING OF FORMS OR THE DELIVERY OF CONCRETE TO THE JOBSITE.
- D. STRUCTURAL STEEL:
- 1) SPECIAL INSPECTIONS OF STEEL CONSTRUCTION SHALL BE PER CBC SECTION 1705.2, CBC TABLE 1705A.2., AND CBC SECTION 12.2
- STRUCTURAL STEEL SPECIAL INSPECTIONS AND NON-DESTRUCTIVE TESTING SHALL BE PER AISC 360 CHAPTER N, AISC 341 CHAPTER J, AISC 358.
- 3) COLD-FORMED STEEL DECK SHALL HAVE SPECIAL INSPECTIONS IN ACCORDANCE WITH QA INSPECTION REQUIREMENTS OF SDI QA/QC.
- 4) OPEN-WEB JOISTS SHALL HAVE SPECIAL INSPECTIONS PER CBC SECTION 1705.2.3 AND TABLE 1705.2.3.
- 5) NON-STRUCTURAL COMPONENTS SHALL BE INSPECTED PER CBC SECTION 1705.13.2.

STRUCTURAL WELDING SHALL BE INSPECTED PER CBC TABLE 1705A.2.1.

7) THE SPECIAL INSPECTOR NEED NOT BE CONTINUOUSLY PRESENT DURING THE WELDING OF THE FOLLOWING ITEMS, PROVIDED THE MATERIALS, QUALIFICATIONS OF WELDING PROCEDURES, AND WELDERS ARE VERIFIED PRIOR TO THE START OF WORK. PERIODIC INSPECTIONS ARE MADE OF WORK

- IN PROGRESS. VISUAL INSPECTION OF ALL WELDS IS MADE PRIOR TO COMPLETION OR PRIOR TO SHIPMENT OF SHOP WELDING.
- E. MASONRY: SPECIAL INSPECTIONS AND TESTS OF MASONRY ELEMENTS SHALL BE IN ACCORDANCE WITH TMS 402/602 TABLE 3 AND TABLE 4 LEVEL 3 QUALITY ASSURANCE REQUIREMENTS.
- F. WOOD: SPECIAL INSPECTIONS OF WOOD STRUCTURAL ELEMENTS SHALL BE IN ACCORDANCE WITH CBC SECTIONS 1705.5, 1705.11.1 AND 1705.12.2.
- 1) SPECIAL INSPECTION OF PREFABRICATED WOOD STRUCTURAL ELEMENTS AND ASSEMBLIES SHALL BE IN ACCORDANCE WITH SECTION 1704.2.5.
- 2) HIGH LOAD DIAPHRAGMS PER CBC SECTION 1705.5.1.
- 3) METAL PLATE CONNECTED WOOD TRUSSES PER CBC SECTION 1705.5.2.
- 4) MANUFACTURE RED TRUSSES PER CBC SECTION 1705.5.3.
- 5) STRUCTURAL GLULAM TIMBER PER CBC SECTION 1705.5.4.
- 6) MANUFACTURED OPEN WEB TRUSSES PER CBC SECTION 1705.5.5
- G. DRIVEN DEEP FOUNDATIONS: SPECIAL INSPECTIONS AND TESTS SHALL BE PER CBC SECTION 1705.7 AND CBC TABLE 1705.7, AND MEET REQUIREMENTS OF APPROVED GEOTECHNICAL REPORT.
- H. CAST-IN-PLACE DEEP FOUNDATIONS: SPECIAL INSPECTIONS AND TESTS SHALL BE PER CBC SECTION 1705.8 AND TABLE 1705.8, AND MEET REQUIREMENTS OF APPROVED GEOTECHNICAL REPORT.
- HELICAL PILE FOUNDATIONS: SPECIAL INSPECTIONS AND TESTS SHALL BE PER CBC SECTION 1705.9 AND MEET REQUIREMENTS OF APPROVED GEOTECHNICAL REPORT.
- J. COLD-FORMED STEEL: PERIODIC SPECIAL INSPECTION IS REQUIRED FOR WELDING OPERATIONS, SCREW ATTACHMENT, BOLTING, ANCHORING AND OTHER FASTENING OF THE SEISMIC FORCE RESISTING SYSTEM PER CBC SECTIONS 1705.11.2 AND 1705.12.3.
- K. ARCHITECTURAL COMPONENTS: PERIODIC SPECIAL INSPECTION IS REQUIRED FOR THE ERECTION AND FASTENING OF EXTERIOR CLADDING, INTERIOR AND EXTERIOR NON-BEARING WALLS AND INTERIOR AND EXTERIOR VENEER, AND ACCESS FLOORS PER CBC SECTION 1705.12.5
- L. MECHANICAL COMPONENTS: PERIODIC SPECIAL INSPECTION OF PLUMBING, MECHANICAL, AND ELECTRICAL COMPONENTS SHALL BE REQUIRED FOR ANCHORAGE AND BRACING PER CBC SECTION
- M. STORAGE RACKS: PERIODIC SPECIAL INSPECTION IS REQUIRED FOR ANCHORAGE OF STORAGE RACKS THAT ARE 8 FEET OR GREATER IN HEIGHT PER CBC SECTION 1705.12.7.
- N. SPRAYED FIRE-RESISTANT MATERIALS: SPECIAL INSPECTIONS AND TESTS OF SPRAYED FIRE-RESISTANT MATERIALS APPLIED TO FLOOR, ROOF, WALL ASSEMBLIES, AND STRUCTURAL MEMBERS SHALL BE PERFORMED PER CBC SECTION 1705.14.
- SHOTCRETE: ALL SHOTCRETE WORK SHALL BE CONTINUOUSLY INSPECTED DURING PLACING BY AN APPROVED AGENCY PER CBC SECTION 1705.19.

#### 9. CBC 2019 (TABLE 1705.6 - SOILS SPECIAL INSPECTION)

# TYPE CONTINUOUS PERIODIC VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTED FILL. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE

#### 10. CBC 2019 (TABLE 1705.3 - CONCRETE SPECIAL INSPECTION)

HAS BEEN PREPARED PROPERLY.

#### TABLE 1705.3 REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION

	TYPE	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCI
	INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	_	x	ACI 318 Ch. 20, 25.2, 25.3, 26.6.1-26.6.3	1908.4
	REINFORCING BAR WELDING: VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706:	-	x	AWS D1.4	
	INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"; AND		X	ACI 318: 26.5.4	_
C.	INSPECT ALL OTHER WELDS.	X			
3.	INSPECT ANCHORS CAST IN CONCRETE.	_	Х	ACI 318: 17.8.2	-
A.	INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.A.	x	x	ACI 318: 17.8.2.4 ACI 318: 17.8.2	_
5.	VERIFY USE OF REQUIRED DESIGN MIX.	_	x	ACI 318: Ch. 19, 26.4.3, 26.4.4	1904.1, 1904.2 1908.2, 1908.3
	PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	х		ASTM C172 ASTM C31 ACI 318: 26.5, 26.12	1908.10
	INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	x	_	ACI 318: 26.5	1908.6, 1908.7 1908.8
	VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	_	x	ACI 318: 26.5.3-26.5.5	1908.9
	INSPECT PRESTRESSED CONCRETE FOR: APPLICATION OF PRESTRESSING FORCES; AND GROUTING OF BONDED PRESTRESSING TENDONS.	X X	_	ACI 318: 26:10	_
	NSPECT ERECTION OF PRECAST CONCRETE MEMBERS.	-	х	ACI 318: 26.9	_
S	ERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO TRESSING OF TENDONS IN POST TENSIONED ONCRETE AND PRIOR TO REMOVAL OF SHORES AND ORMS FROM BEAMS AND STRUCTURAL SLABS.	_	x	ACI 318: 26.11.2	_
A	NSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	_	x	ACI 318: 26.11.2(b)	_

WHERE APPLICABLE, SEE SECTION 1705.12, SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE.

SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION SHALL BE INCLUDED IN THE RESEARCH REPORT OF

b. SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION SHALL BE INCLUDED IN THE RESEARCH REPORT FOR THE ANCHOR ISSUED BY AN APPROVED SOURCE IN ACCORDANCE WITH 17.8.2 IN ACI 318, OR OTHER QUALIFICATION PROCEDURES. WHERE SPECIFIC REQUIREMENTS ARE NOT PROVIDED, SPECIAL INSPECTION REQUIREMENTS SHALL BE SPECIFIED BY THE REGISTERED DESIGN PROFESSIONAL AND SHALL BE APPROVED BY THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF THE WORK.

C. [OSHPD 1R, 2 & 5] INSTALLATION OF ALL ADHESIVE ANCHORS IN HORIZONTAL AND UPWARDLY INCLINED POSITIONS SHALL BE PERFORMED BY AN ACI/CRSI CERTIFIED ADHESIVE ANCHOR INSTALLER, EXCEPT WHERE THE FACTORED DESIGN TENSION ON THE ANCHORS IS LESS THAN 100 POUNDS AND THOSE ANCHORS ARE CLEARLY NOTED ON THE APPROVED CONSTRUCTION DOCUMENTS OR WHERE THE ANCHORS ARE SHEAR DOWELS ACROSS COLD JOINTS IN SLABS ON GRADE WHERE THE SLAB IS NOT PART OF THE LATERAL FORCE-RESISTING SYSTEM.

#### 11. CBC 2019 (TABLE 1705A.2.1 - STEEL SPECIAL INSPECTION

#### TABLE 1705A.2.1 REQUIRED SPECIAL INSPECTIONS AND TESTS OF STEEL CONSTRUCTION

CONTINUOUS DEPLODIC

		CONTINUOUS	PERIODIC	0555554055	
	TYPE	SPECIAL	SPECIAL	REFERENCED	CBC
		INSPECTION	INSPECTION	STANDARD <sup>a</sup>	REFERENCE
1.	MATERIAL IDENTIFICATION AND TESTING OF HIGH-STRENG	TH BOLTS, NUTS A	ND WASHERS:	Annual Control of the	
A.			T	RCSC:1.5;	2202A.1,
	STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	_	×	AISC360: A3.3, J3.1 AND APPLICABLE ASTM MATERIAL STANDARDS	[DSA-SS/CC] 2202.1
B.	MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	<u>-</u>	х	RCSC: 1.5 & 2.1; AISC360: A3.3 & N3.2	_
C.	TESTING OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS.	_	_	RCSC: 7.2, APPLICABLE ASTM MATERIAL STANDARDS	2213A.1, [DSA-SS/CC 2212.6.1
2.	INSPECTION OF HIGH-STRENGTH BOLTING:	**************************************	L	01111071100	
A.	SNUG-TIGHT JOINTS.		X		
B.	PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITH MATCHMARKING, TWIST-OFF BOLT OR DIRECT TENSION INDICATOR METHODS OF INSTALLATION.	_	x	RCSC: 7-9 AISC360: J3.1, J3.2,	1705A.2.6, 2204A.2,
C.		х	_	M2.5 & N5.6	[DSA-SS/CC] 2204.2
3.	MATERIAL IDENTIFICATION AND TESTING OF STRUCTURAL	STEEL AND COLD-	FORMED STEEL DE	CK:	
A.	FOR STRUCTURAL STEEL, IDENTIFICATION MARKINGS TO CONFORM TO AISC360.	_	х	AISC360: A3.1	2202A.1, [DSA-SS/CC] 2202.1
B.	FOR OTHER STEEL, IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	х	APPLICABLE ASTM MATERIAL STANDARDS	2202A.1, [DSA-SS/CC] 2202.1
C.	MANUFACTURER'S CERTIFIED TEST REPORTS.		X	AISC360: A3.1 & N3.2	2202.1
D.	TESTING OF UNIDENTIFIED STEEL	-	_	APPLICABLE ASTM MATERIAL	2202A.1, [DSA-SS/CC]
	MATERIAL IDENTIFICATION OF MELBING CONSUMANTES AN		L	STANDARDS	2202.1
4.	MATERIAL IDENTIFICATION OF WELDING CONSUMABLES AN	D TESTING OF WE	LDED ELEMENTS:		
A.	IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THE APPROVED CONSTRUCTION DOCUMENTS.	· · · - · · · ·	x	AISC360, A3.5 & N3.2 AND APPLICABLE AWS A5 DOCUMENTS	_
B.	MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	-	х	AISC360: N3.2	-
C.	NONDESTRUCTIVE TESTING OF WELDED JOINTS.		_	AISC360: N5.5	
5.	INSPECTION OF WELDING:		***************************************		***************************************
A.	STRUCTURAL STEEL AND COLD-FORMED STEEL DECK:				
	COMPLETE AND PARTIAL JOINT PENETRATION     GROOVE WELDS	X	_	AICC260	
-	MULTIPASS FILLET WELDS	X		AISC360: J2, M2.4, & M4.5,	1705A.2.1,
_	3. SINGLE-PASS FILLET WELDS > 5/16"	X	_	AWS D1.1	1705A.2.1
	4. PLUG AND SLOT WELDS	X		AWS D1.8	
	<ol> <li>SINGLE-PASS FILLET WELDS ≤ 5/16"</li> </ol>	-	X		
	FLOOR AND ROOF DECK WELDS.		х	AWS D1.3, SDI QA/QC	1705A.2.1, 1705A.2.2, 1705A.2.5
	7. END-WELDED STUDS.	_	x	AWS D1.1	1705A.2.5, 2213A.2, [DSA-SS/CC] 2212.6.2
	8. WELDED SHEET STEEL FOR COLD-FORMED FRAMING MEMBERS	_	X	AWS D1.3	1705A.2.5, 1705A.2.4.1
B.	REINFORCING STEEL:				
	VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A706.	-	х		
	REINFORCING STEEL RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL STRUCTURAL WALLS OF CONCRETE AND SHEAR REINFORCEMENT.	x	_	AWS D1.4, ACI 318: 18.2.8, 25.5.7.4, 26.6.4.1	1705A.3.1, 1903A.8
_	3. SHEAR REINFORCEMENT	X			
_	OTHER REINFORCING STEEL.		X		
	5. TESTS OF REINFORCING BARS.	, -	_	_	1910A.2, [DSA-SS/CC] 1909.2.4
6.	INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIA	NCE:			1000.2.4
_	A. DETAILS SUCH AS BRACING AND STIFFENING.	_	Х	T	
	B. MEMBER LOCATIONS.		X	AISC360:	1705A.2.1
-	C. APPLICATION OF JOINT DETAILS AT EACH CONNECTION.	_	х	N5.8	1700A.Z.1

#### 12. CBC 2019 (TABLE 1705.8 - DEEP FOUNDATION - CIP)

#### TABLE 1705.8 REQUIRED SPECIAL INSPECTIONS AND TESTS OF CAST-IN-PLACE DEEP FOUNDATION ELEMENTS

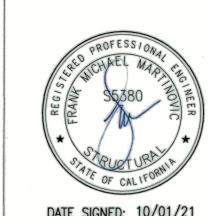
	TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1.	INSPECT DRILLING OPERATIONS AND MAINTAIN COMPLETE AND ACCURATE RECORDS FOR EACH ELEMENT.	х	_
2.	VERIFY PLACEMENT LOCATIONS AND PLUMBNESS, CONFIRM ELEMENT DIAMETERS, BELL DIAMETERS (IF APPLICABLE), LENGTHS, EMBEDMENT INTO BEDROCK (IF APPLICABLE) AND ADEQUATE EN-BEARING STRATA CAPACITY. RECORD CONCRETE OR GROUT VOLUMES.	х	_
3.	FOR CONCRETE ELEMENTS, PERFORM TESTS AND ADDITIONAL SPECIAL INSPECTIONS IN ACCORDANCE WITH SECTION 1705.3.	_	_

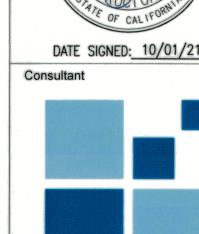
Architect

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City Approvals

Approved by Date

Issue: 100% CDs W/ PLAN CHECK CORRECTIONS 9-20-2021

No. Date Description

McNair Soccer Complex Phase - 2

9550 Ronald E. McNair Way Stockton CA 95210

Architect of Record JH
Project Architect JH
Drafted By ATL
Checked By FM
File Date

GENERAL NOTES

**Sheet Title** 

**Project Number** 

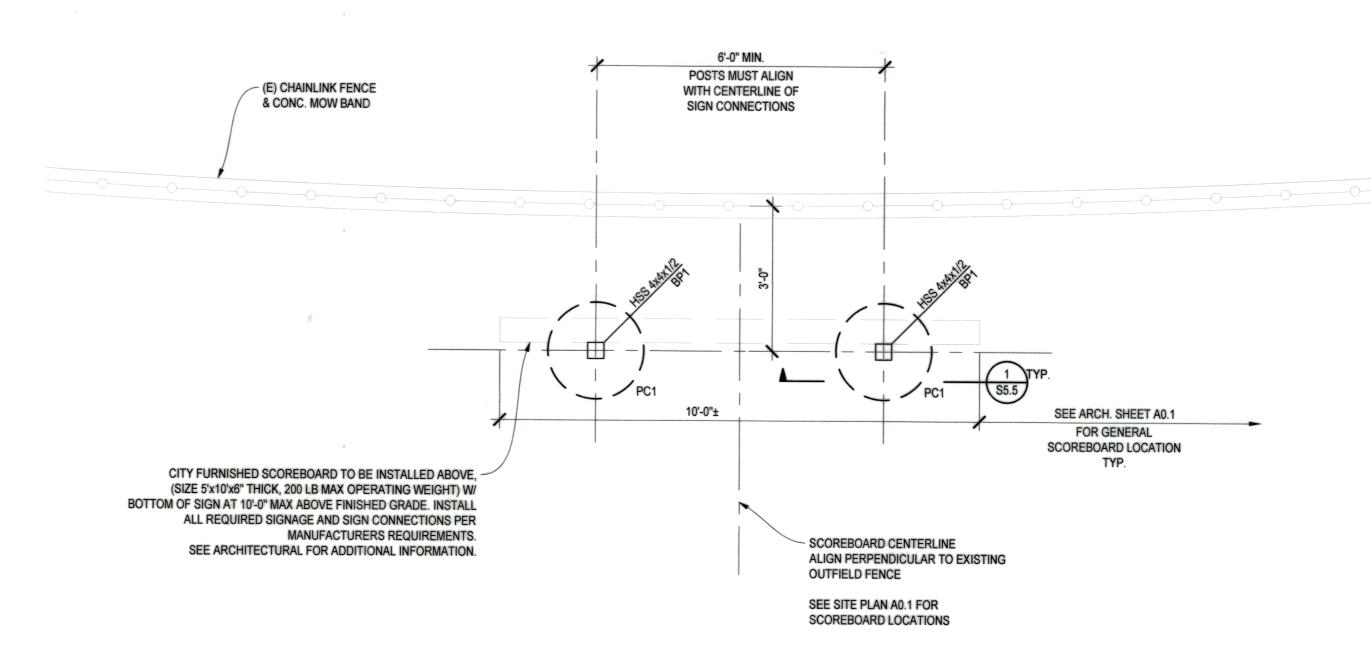
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Sheet Scale

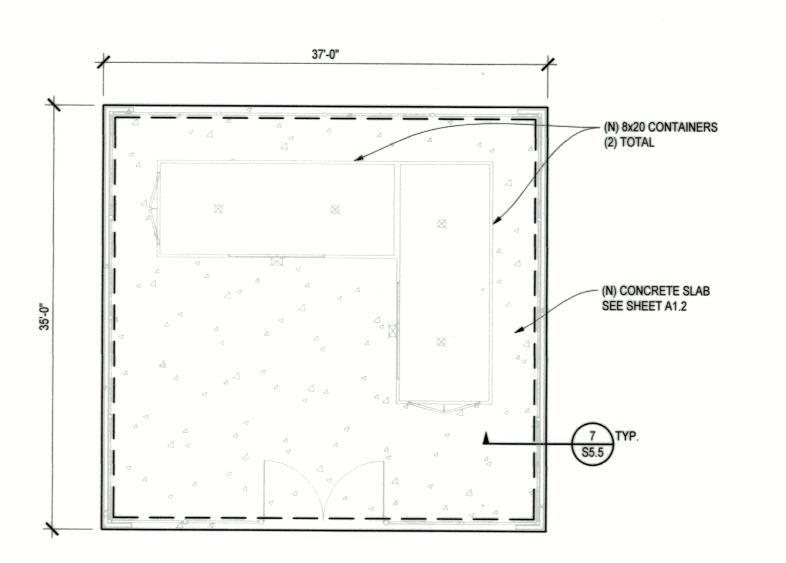
Sheet Number

S1.1

SOFTBALL FIELD INTERIOR



2 SCOREBOARD PARTIAL FOUNDATION PLAN



### FOUNDATION NOTES:

- TOP OF FINISHED GRADE = 100'-0" REFERENCE ELEVATION, SEE CIVIL DRAWINGS FOR ACTUAL ELEVATIONS. SEE DETAILS 8, 9, & 10 ON S5.1 FOR UTILITY REQUIREMENTS AROUND THE BUILDING FOUNDATIONS TYPICAL.
- CONCRETE SLAB ON GRADE SHALL BE 6" THICK MINIMUM WITH #4 AT 12" OC EACH WAY PLACED AT 1
  1/2" FROM TOP OF SLAB. FOR SLAB CONSTRUCTION SEE DETAIL 8 ON S5.1. PROVIDE CONSTRUCTION
  AND CONTROL JOINTS, SUBMIT JOINT LAYOUT DRAWING TO ARCHITECT AND ENGINEER FOR
  APPROVAL. SEE ARCHITECTURAL DRAWINGS FOR SLAB SLOPES, DEPRESSIONS, CURBS,
  HOUSEKEEPING PADS, WATERPROOFING REQUIREMENTS.
- TOP OF FOOTING SHALL BE AT ELEVATION 99'-0" REFERENCE TO FINISHED GRADE UNLESS NOTED OTHERWISE T/F=XX'-XX" ON THE PLAN.
- PC1 = 24" DIAMETER POLE FOUNDATION PER DETAIL 1 ON S5.5.

  BP1 = BASEPLATE 12"x12"x 1 1/8" (A36) WITH (4) 3/4" DIA. F1554-36 ANCHOR BOLTS WITH 27" EMBED PER DETAILS 3 & 4 ON S5.5.
- CONTRACTOR TO FIELD VERIFY (E) FRAMING & FOUNDATIONS AS REQUIRED INFORMATION SHOWN IS BASED ON FIELD OBSERVATION. NOTIFY SEOR WHERE CONDITIONS ARE DIFFERENT FROM NOTED.
- SEE GENERAL NOTES SHEETS \$1.0 THRU \$1.2 FOR ADDITIONAL MATERIAL AND DESIGN REQUIREMENTS NOTED STATED IN THESE PLAN NOTES. SPECIAL INSPECTION REQUIREMENT ARE DETAILED IN THE GENERAL NOTES.

Architect

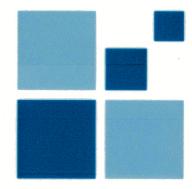
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DATE SIGNED: 10/01/21

Consultant



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City Approvals

Public Works Department

Approved by Date

Issue: 100% CDs W/ PLAN CHECK CORRECTIONS 9-20-2021

	No.	Date	Description
_			
8			
_			
	Proje	ct	
9			

McNair Soccer Complex Phase - 2

9550 Ronald E. McNair Way Stockton CA 95210

Architect of Record	JH
Project Architect	JH
Drafted By	ATL
Checked By	FM

Sheet Title

PARTIAL PLAN-STRUCTURAL

Project Number
CR 16024

Reference
North
Sheet Scale

Sheet Number

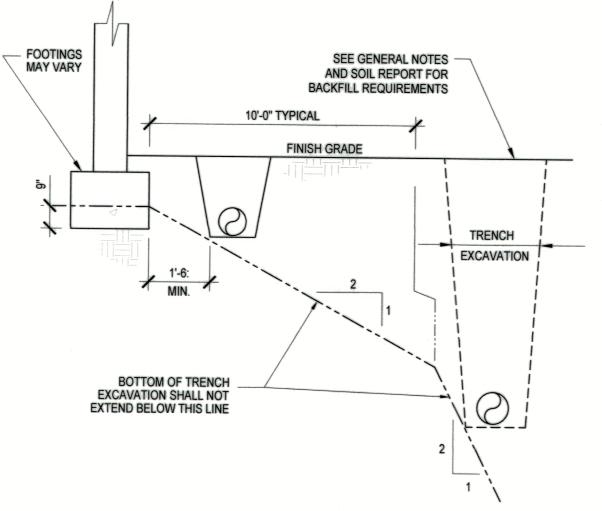
3 CONTAINER SLAB FOUNDATION PLAN

5463.7C

NOTES:

1. FOR PIPES 3'-0" OR LESS BELOW BOTTOM OF FOOTINGS PROVIDE SLEEVE AND CONCRETE AS SHOWN. FIRE SERVICE LINES SHALL HAVE 2" MIN. CLR. ALL AROUND TRENCHES FOR PIPES/CONDUITS WITH INVERT ELEVATION GREATER THAN SHOWN SHALL BE BACKFILLED PER GEOTECH RECOMMENDATIONS.

**EXCAVATION PERPENDICULAR TO FOOTING** 



K

THE CONTRACTOR SHALL COORDINATE ALL EXCAVATION OPERATIONS WITH BUILDING FOUNDATION REQUIREMENTS.

M

## **EXCAVATION PARALLEL TO NEW FOUNDATION**

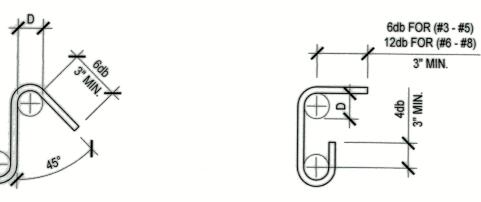
CONCRETE MEMBER REINFORCEMENT EXPOSURE		REINFORCEMENT	SPECIFIED COVER (IN UNO	
CAST AGAINST AND PERMANENTLY IN CONTACT WITH GROUND	ALL	ALL	3	
EXPOSED TO WEATHER OR IN CONTACT WITH GROUND	ALL	#6 THROUGH #18 BARS	2	
CONTACT WITH GROUND		#5 BARS AND SMALLER 1-1/	1-1/2	
	SLAB, JOIST, AND	#14 AND #18 BARS	1-1/2	
NOT EXPOSED TO WEATHER	WALLS	#11 BARS AND SMALLER	3/4	
OR IN CONTACT WITH GROUND	BEAMS, COLUMNS, PEDESTALS AND TENSION TIES	PRIMARY REINFORCEMENT, STIRRUPS, TIES, SPIRALS, AND HOOPS	1-1/2	
	MAIN RE	INF. (INSIDE/OUTSIDE FACE)	1	
TILT UP PANELS	TIES/STIR	3/4		
TIET OF FAMELO	MAIN	REINF. EDGE OF PANEL	2	
	TIES/S	STIRRUPS EDGE OF PANEL	1-1/2	

NOTES:

- MINIMUM SPECIFIED COVER ABOVE IS BASED ON CAST-IN-PLACE CONSTRUCTION (NON-PRESTRESSED).
- PROVIDE 1-1/2" CLEAR FROM TOP OF SLAB FOR SLAB ON GRADE. FOR PRECAST, PRE-STRESSED USE ALTERNATE SCHEDULE.
- 4. REINFORCING BARS SHALL EXTEND AS FAR AS POSSIBLE AND END IN EITHER A STANDARD HOOK OR TENSION LAP SPLICE

UNLESS DETAILED OTHERWISE.

## CONCRETE CLEAR COVER



STIRRUPS & TIES

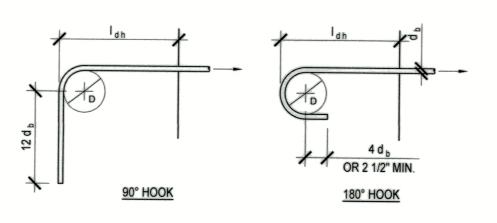
SEISMIC STIRRUPS & TIES

INSIDE DIAMETER (D) FOR TIE BENDS D=1-1/2" FOR #3 D=2" FOR #4 D=2-1/2" FOR #5 D=6db FOR #6 - #8 180° HOOK -4 x db OR 3" MIN.

**BAR BENDING** 

INSIDE DIAMETER (D) FOR ALL BENDS 6 BAR DIA. (db) FOR #3 - #8 8 BAR DIA. (db) FOR #9 - #11 10 BAR DIA. (db) FOR #14 & #18

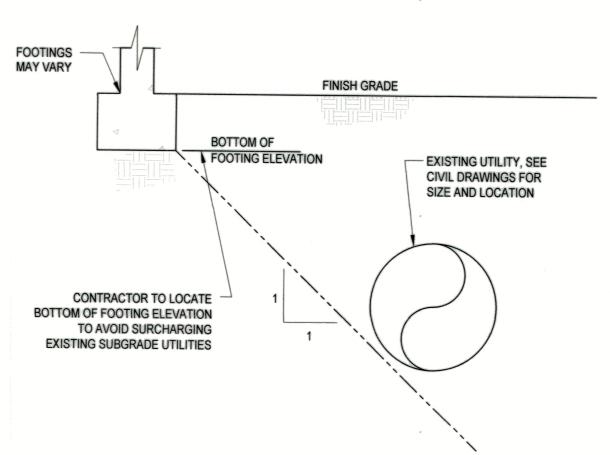
STANDARD HOOK



**GENERAL USE** 1. SIDE COVER > 2 1/2 IN. ANCHOR BOLTS 2. END COVER (90° HOOKS) > 2 IN.

BAR	NORMAL WEIGHT CONCRETE, fc (PSI)										
SIZE NO.	3,000	3,500	4,000	5,000	6,000						
#3	9	8	8	7	6						
#4	11	11	10	9	8						
#5	14	13	12	11	10						
#6	17	16	15	13	12						
#7	20	18	17	15	14						
#8	22	21	19	17	16						
#9	25	23	22	20	18						
#10	28	26	25	22	20						
#11	31	28	27	24	22						
#14	38	36	33	20	27						

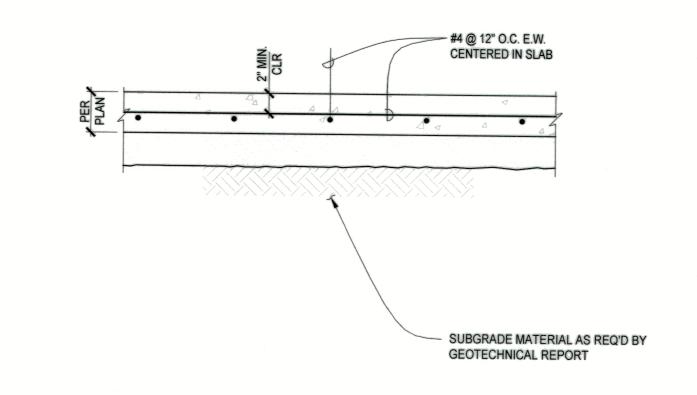
STANDARD HOOK



G

F E

FOOTING PARALLEL TO (E) UTILITY



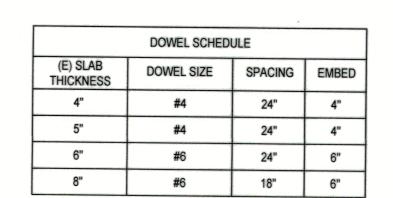
TYPICAL CONCRETE SLAB ON GRADE

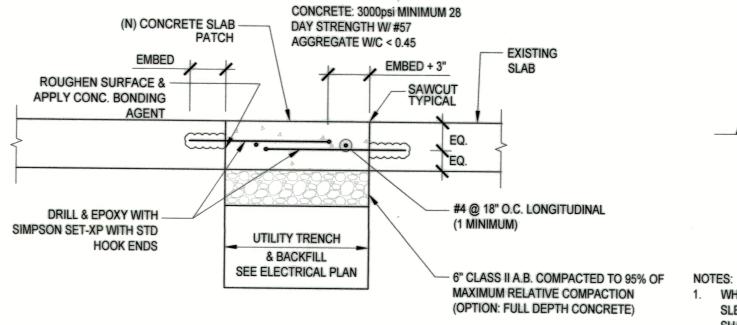
D

		CONCRETE	fc=3000 PS	3000 PSI CONCRETE fc=3500 PSI			CONCRETE fc=4000 PSI				CONCRETE fc=5000 PSI					
	TENSIO	ON LAP	TEN	SION	TENSIO	ON LAP	TEN	SION	TENSION LAP TENSION		TENSION LAP		TENSION			
BAR SIZE	SPL	.ICE	DEVELO	PMENT	SPL	ICE	DEVELO	PMENT	SPI	LICE	DEVELO	PMENT	SPI	ICE	DEVELO	PMENT
SIZE	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER
#3	30	23	23	17	28	21	21	16	26	20	20	15	23	17	17	13
#4	38	29	29	22	37	28	28	21	33	25	25	19	30	23	23	17
#5	49	37	37	28	45	34	34	26	42	32	32	24	38	29	29	22
#6	56	43	43	33	54	41	41	31	50	38	38	29	45	34	34	26
#7	82	63	63	48	77	59	59	45	72	55	55	42	65	50	50	38
#8	94	72	72	55	88	67	67	51	82	63	63	48	73	56	56	43
#9	106	81	81	62	99 .	76	76	58	93	71	71	54	82	63	63	48
#10	119	91	91	70	111	85	85	65	104	80	80	61	93	71	71	54
#11	133	102	102	78	123	94	94	72	115	88	88	67	102	78	78	60
#14	ME		121	93	ME	СН	112	93	ME	СН	112	81	ME	СН	94	72
#18	SPL	ICE	162	124	SPL	ICE	150	124	SPL	ICE.	150	108	SPL		125	96

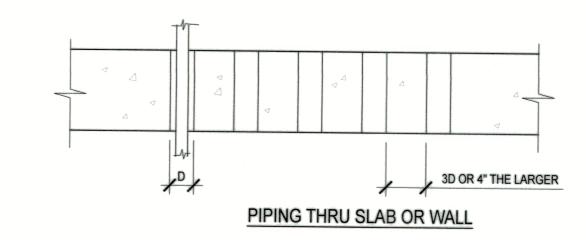
- SPLICE AND DEVELOPMENT LENGTHS ARE GIVEN IN INCHES ASSUMING NORMAL WEIGHT CONCRETE AND GRADE 60 REINFORCING.
- LAP SPLICE AND DEVELOPMENT LENGTHS BASED ON ACI 25.4.2.2, ASSUMING A MINIMUM CLEAR SPACING 2DB, AND MINIMUM CLEAR COVER OF 1DB. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW.
- LAP SPLICES SHALL NOT BE PLACED WITHIN JOINTS, OR WITHIN A DISTANCE OF TWICE THE BEAM DEPTH FROM THE FACE OF THE JOINT
- MECHANICAL SPLICE SHALL BE DAYTON BAR-LOCK COUPLER OR APPROVED EQUAL CAPABLE OF DEVELOPING 125% OF REBAR TENSILE STRENGTH. WHERE LIGHT WEIGHT CONCRETE IS USED INCREASE TENSION LAP AND DEVELOPMENT LENGTHS BY 30%.
- WHERE EPOXY COATED OR ZINC COATED BARS ARE USED INCREASE TENSION LAP AND DEVELOPMENT BY 50%.
- 8. LAP SPLICE AND DEVELOPMENT LENGTHS FOR BUNDLED BARS SHALL BE INCREASED BY 20% FOR 3-BAR BUNDLE, AND 33% FOR A 4-BAR BUNDLE.

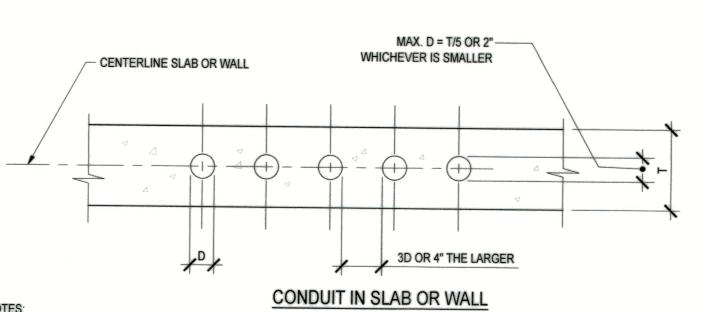
REINFORCING STEEL TENSION DEVELOPMENT & LAP SPLICE





UTILITY TRENCH SLAB REPAIR





 WHERE CLEAR DISTANCE BETWEEN SLEEVES IS IMPOSSIBLE THIS AREA SHALL BE TREATED AS A SLAB OPENING OR AS A WALL OPENING.

CONDUIT IN SLAB OR WALL

HAMMOND+PLAYLE ARCHITECTS, LLP art, architecture + ecology 909 FIFTH STREET, DAVIS, CA 530.750.0756 WWW.INDIGOARCH.COM

DATE SIGNED: 10/01/21 Consultant

SIEGFRIED

3428 Brookside Road Stockton, California 95219 209-943-2021 Fax: 209-942-0214 www.siegfriedeng.com

City Approvals

Issue: 100% CDs W/ PLAN CHECK

		CORRECT	TIONS 9-20-2021
	No.	Date	Description
			,
8			
			,
	Proie	ct	L

**McNair Soccer** Complex Phase - 2

9550 Ronald E. McNair Way Stockton CA 95210

File Date

**FOUNDATION DETAILS & SECTIONS** 

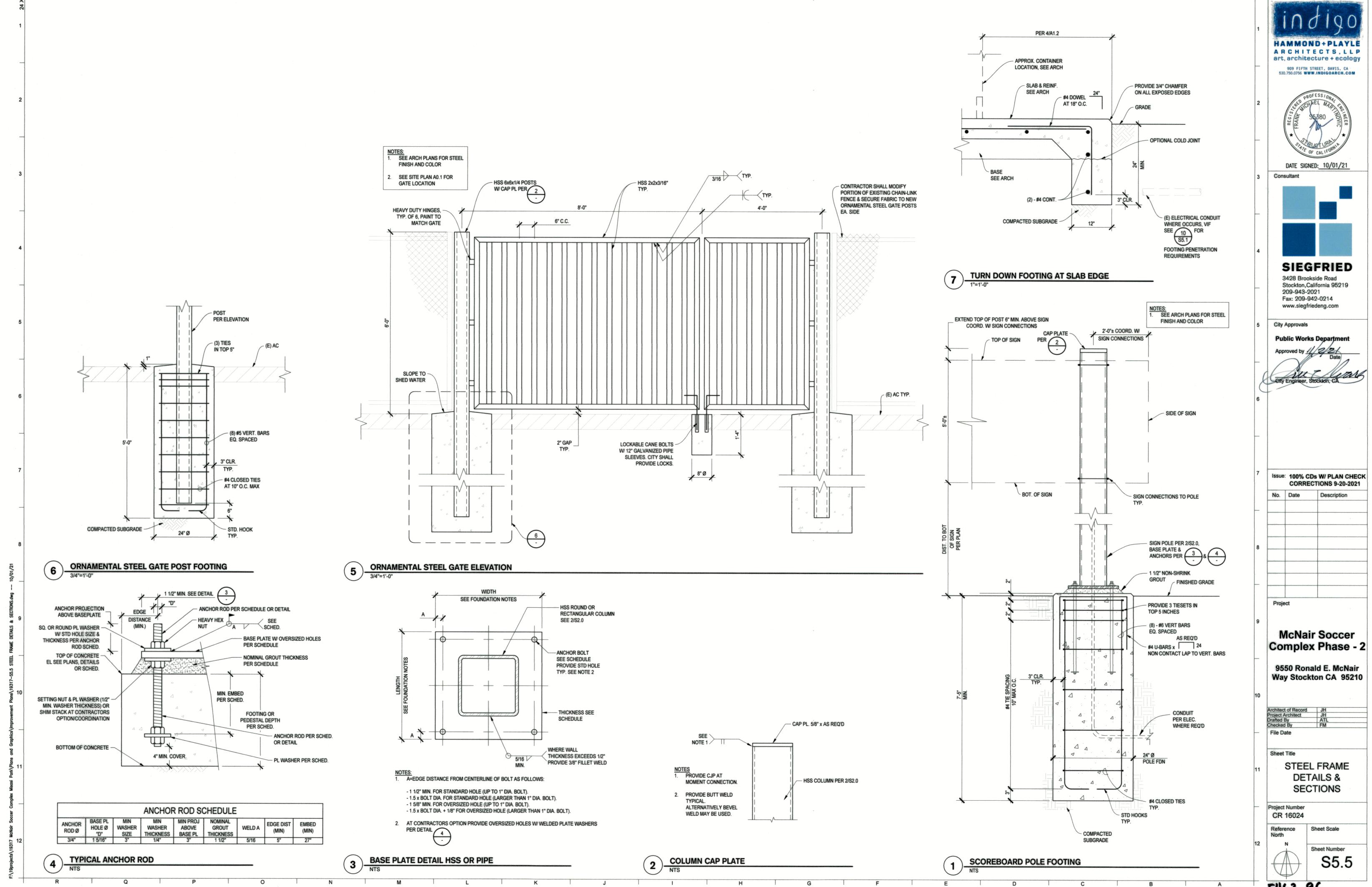
Project Number CR 16024

Reference North

Sheet Number

Sheet Scale

5463. BC



5463.96

	ABBRE	VIATION	IS
Α	AMPERES	MFR.	MANUFACTURER
AC A.F.F.	ALTERNATING CURRENT ABOVE FINISHED FLOOR	NEMA	NATIONAL ELECTRICAL MANUFACTURERS ASSOCIATION
A.I.C.	AMPERE INTERRUPTING CAPACITY	PFB	PROVISIONS FOR FUTURE CIRCUIT BREAKER
AMP	AMPERE	PH	PHASE
AWG	AMERICAN WIRE GAUGE	RCPT.	RECEPTACLE
BKR	BREAKER	S.M.S	SHEET METAL SCREW
C.	CONDUIT	SYS	SYSTEM
C.B.	CIRCUIT BREAKER	TYPICAL	
CKT	CIRCUIT	UGPB	UNDERGROUND PULLBOX
C.O.	CONDUIT ONLY, WITH PULL WIRE	UL	UNDERWRITERS LABORATORY
EMT	ELECTRICAL METALLIC CONDUIT	V	VOLT
GA.	GAUGE	VA	VOLT-AMPERES
GND	GROUND	w	WIRE, WATT
GFCI	GROUND FAULT CIRCUIT INTERRUPTER	WP	WEATHER PROTECTED
Isc	SHORT CIRCUIT AMPERES	XFMR	TRANSFORMER
KVA	KILO VOLT AMPERE	A WIT	TO THE CONTROL OF THE

### APPLICABLE CODES AND REGULATIONS

THE FOLLOWING ARE THE ENFORCEABLE CODES FOR FACILITIES UNDER THE AUTHORITY OF THE ALFRED E. ALQUIST HOSPITAL FACILITIES SEISMIC SAFETY ACT OF 1983:

CALIFORNIA BUILDING STANDARDS ADMINISTRATIVE CODE

(PART 1, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR)).

CALIFORNIA BUILDING CODE - PART 2, TITLE 24, CCR (2015 IBC AND 2016 CALIFORNIA AMENDMENTS).

CALIFORNIA ELECTRICAL CODE - PART 3, TITLE 24, CCR (2014 NEC AND 2014 CALIFORNIA AMENDMENTS).

CALIFORNIA MECHANICAL CODE - PART 4, TITLE 24, CCR (2015 UMC AND 2016 CALIFORNIA

CALIFORNIA PLUMBING CODE - PART 5, TITLE 24, CCR (2015 UPC AND 2016 CALIFORNIA AMENDMENTS).

CALIFORNIA FIRE CODE - PART 9, TITLE 24, CCR (2015 IFC AND 2016 CALIFORNIA AMENDMENTS)

CALIFORNIA GREEN BUILDING STANDARDS CODE - PART 11. TITLE 24

2015 N.F.P.A. 101 LIFE SAFETY CODE

2015 N.F.P.A. 99 STANDARD FOR HEALTH CARE FACILITIES

N.F.P.A. 72 NATIONAL FIRE ALARM CODE

EDITION TITLE 19, CALIFORNIA CODE OF REGULATIONS, PUBLIC SAFETY, STATE FIRE MARSHAL

TITLE 8, CALIFORNIA CODE OF REGULATIONS, INDUSTRIAL STANDARDS.

TITLE 22, CALIFORNIA CODE OF REGULATIONS, SOCIAL SECURITY DIVISIONS 5, 7.

THE OFFICE OF STATEWIDE HEALTH PLANNING AND DEVELOPMENT (OSHPD) REGULATIONS.

OCCUPATIONAL SAFETY AND HEALTH ACT (OSHA).

FACTORY MUTUAL SYSTEM (FM) REQUIREMENTS. ALL APPLICABLE STATE AND LOCAL CODES AND REGULATIONS

#### ELECTRICAL SYMBOL LIST

LUMINAIRE, WITH POLE AND BASE

SINGLE POLE TOGGLE SWITCH, +45" A.F.F. - "a" LETTER DENOTES SWITCH FUNCTION, TYPICAL FOR ALL SWITCHES UNLESS NOTED OTHERWISE

JUNCTION BOX - SIZE AS REQUIRED BY CODE

NEW UNDERGROUND PULL BOX

EXISTING CONDUIT AND WIRING

— — CONDUIT RUN UNDERFLOOR OR UNDERGROUND MINIMUM 1" DIAMETER.

CONDUIT HOMERUN TO PANELBOARD

CONDUIT TURNED AND RISED UP

CONDUIT TURNED AND DROPPED DOWN

CONDUIT WITH CAP

CONDUIT STUB WITH INSULATED BUSHING

DRAWING SHEET NUMBERED NOTE DESIGNATION - APPLIES TO NUMBERED NOTE ON SAME SHEET

DRAWING PLAN OR DETAIL DESIGNATION - "1" OR "A" DENOTES PLAN OR DETAIL NUMBER, E-1 "E-1" DENOTES SHEET NUMBER

FEEDER TAG, DENOTES SIZE OF CONDUIT AND WIRE. SEE FEEDER SCHEDULE.

#### SYMBOL LIST NOTES:

- EXISTING ELECTRICAL EQUIPMENT, OUTLETS, AND DEVICES ARE SHOWN THE SAME AS NEW, EXCEPT LIGHTLY AND ACCOMPANIED BY (E). SUCH ELECTRICAL EQUIPMENT, OUTLETS, AND DEVICES ARE TO REMAIN AS IS, UNLESS OTHERWISE NOTED ON PLAN OR SPECIFICATION.
- VERIFY ON SITE THAT ALL PANELBOARDS HAVE MINIMUM WORKING SPACES PER CODE AND THAT THE DEDICATED PANELBOARD SPACES ARE CLEAR OF ALL DUCTS, PIPING AND EQUIPMENT FOREIGN TO THE PANEL BOARDS. NOTIFY THE ENGINEER FOR CORRECTIVE ACTION IN THE EVENT THAT FOREIGN OBJECTS IMPEDE THE DEDICATED PANELBOARD AREAS.
- WHERE CONDUIT STUB IS INDICATED, PROVIDE CONDUIT WITH BUSHING AT THE END OF CONDUIT AND PULL ROPE INTO ACCESSIBLE CEILING AREA.

		LUMINAIRE SCH	HEDULE		
TYPE	MANUFACTURER	VOLTAGE	LIGHT SOURCE (LED, WATTS, LUMENS,	MOUNTING	REMARK NOTE No.
TIPE	CATALOG NO.	DESCRIPTION	COLOR TEMPERATURE, CRI, R9 IF AVAILABLE)		
^	GUARDCO SIGNIFY	12V SOLAR POWERED	LED, 17W, 2000L,	POLE MOUNT	
Α	BRP710-LED-WW-MR- 35MO-PDIM50-SOLAR	LED FLOODLIGHT	4000K°		
-	EATON ALL-PRO LED	12V SOLAR POWERED			
В	MST800L	LED FLOODLIGHT	LED, 90W, 1000L	SURFACE	
-	EATON ALL-PRO LED	4" STRAIGHT STEEL SQ	N/A		
Р	SSS12-4-11-T2X6	POLE, 110 MPH RATED			

#### LUMINAIRE SCHEDULE REMARK NOTES:

GENERAL NOTE:

REFER TO PLAN FOR LOCATION, QUANTITIES, AND SWITCH FUNCTION.

		ELECTRICAL SHEET INDEX
No. OF SHEETS	DRAWING No.	DRAWING DESCRIPTIONS
1	E0.1	ELECTRICAL SHEET INDEX, SYMBOL LIST, AND ABBREVIATIONS
2	E1.0	ENLARGED SITE PLAN - POWER AND LIGHTING
3	E1.1	ENLARGED SITE PLAN - ELECTRICAL
4	E2.0	ONE-LINE POWER DIAGRAM, PANEL SCHEDULE, AND DETAILS

#### \*\* UNDERGROUND DIGGING CAUTION \*\*

USE EXTREME CAUTION WHEN DIGGING TO AVOID BURIED UTILITY CABLES, CONDUITS, AND PIPING. CALL "UNDERGROUND SERVICE ALERT" (U.S.A.):

1-800-642-2444

TWO WORKING DAYS BEFORE DIGGING TO VERIFY UNDERGROUND UTILITIES.

#### BRANCH CIRCUIT VOLTAGE DROP TABLE

VOLTAGE DROP VALUES NOT EXCEEDING 2% FOR FEEDERS HAVE BEEN SHOWN ON THE ONE LINE DIAGRAM. IN LIEU OF VOLTAGE DROP CALCULATIONS FOR EACH BRANCH CIRCUIT IN SCOPE OF WORK, THE GENERAL DESIGN STANDARD BELOW HAS BEEN FOLLOWED TO ENSURE A VOLTAGE DROP OF 3% IS NOT EXCEEDED. WHERE THE CIRCUIT LOAD OR CIRCUIT LENGTH LISTED HAS BEEN EXCEEDED, A DETAILED VOLTAGE DROP CALCULATION FOR THAT CIRCUIT HAS BEEN PROVIDED WITH THE TITLE 24 COMPLIANCE DOCUMENTATION.

VOLTAGE	MAXIMUM CIRCUIT LOAD FOR 20 AMP CIRCUIT BREAKER	CONDUCTOR SIZE	MAXIMUM BRANCH CIRCUIT LENGTH
		#8 AWG	285 FEET
	9 AMPS (1.08 KVA)	#10 AWG	180 FEET
		#12 AWG	115 FEET
		#8 AWG	210 FEET
120 VOLT	12 AMPS (1.44 KVA)  16 AMPS (1.92 KVA)	#10 AWG	135 FEET
		#12 AWG	85 FEET
		#8 AWG	130 FEET
		#10 AWG	85 FEET
		#12 AWG	55 FEET
	9 AMPS (2.16 KVA)	#8 AWG	510 FEET
		#10 AWG	330 FEET
		#12 AWG	200 FEET
	12 AMPS (2.88 KVA)  16 AMPS (3.84 KVA)	#8 AWG	380 FEET
240 VOLT		#10 AWG	250 FEET
		#12 AWG	150 FEET
		#8 AWG	285 FEET
		#10 AWG	185 FEET
		#12 AWG	110 FEET



Agency approval

City Approvals

**Public Works Department** 

100% CDs W/ PLAN CHECK CORRECTIONS

Description

**Project** 

McNair Soccer Complex Phase - 2

9820 Ronald E. McNair Way Stockton CA 95210

Sheet Title **ELECTRICAL** 

SHEET INDEX, SYMBOL LIST, AND **ABBREVIATIONS** 

Project Number CR16024

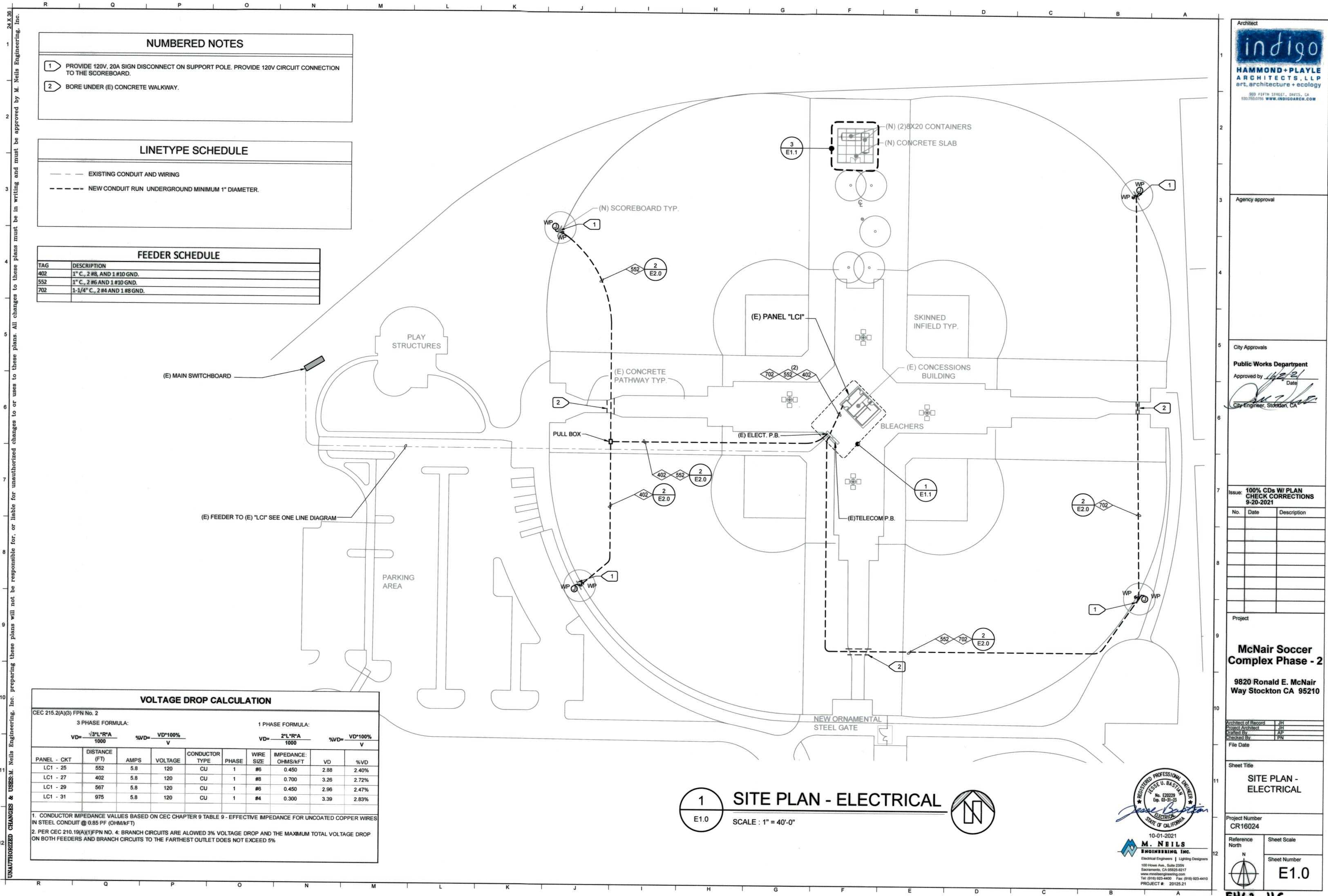
> Reference North

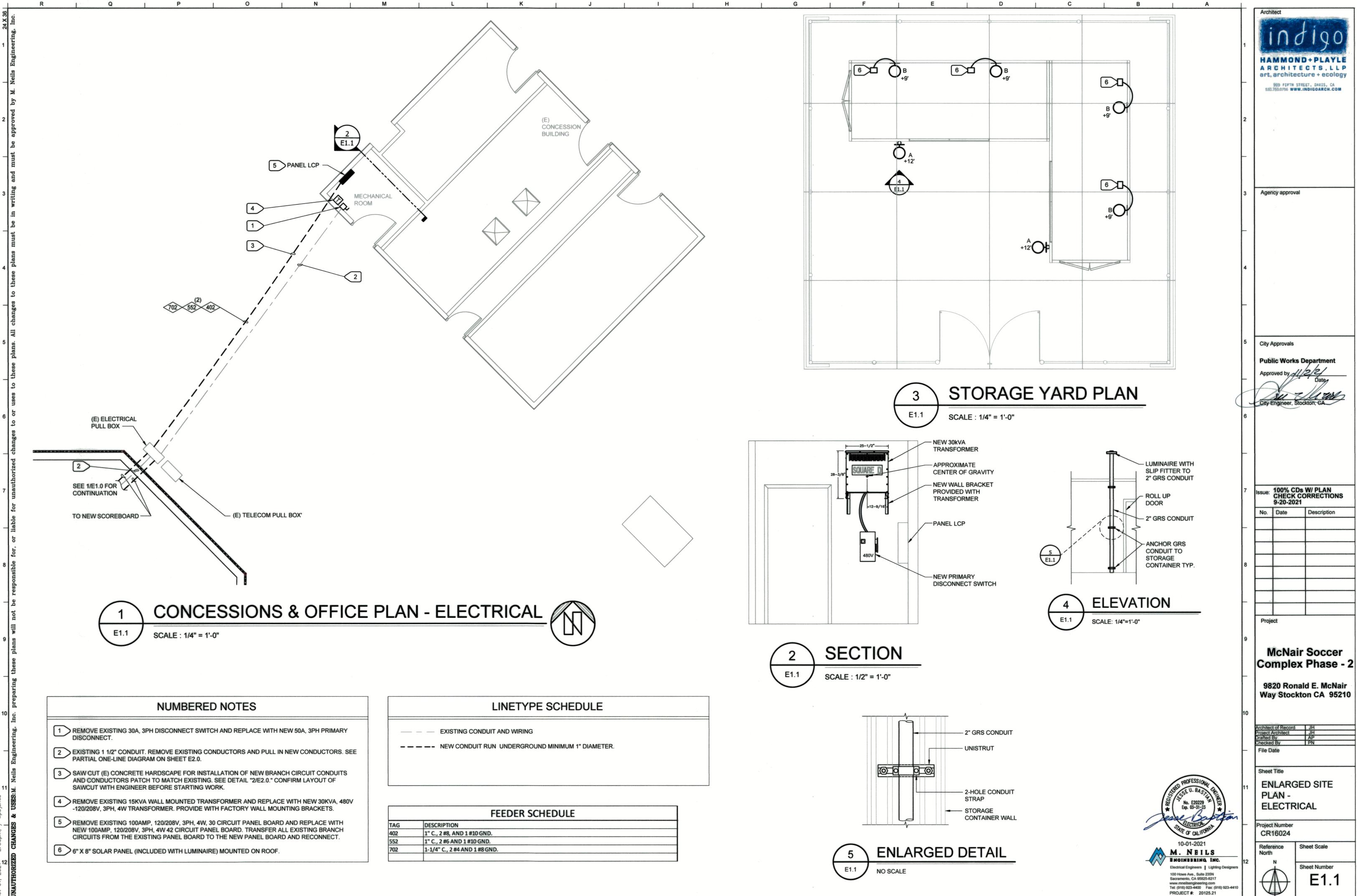
M. NEILS ENGINEERING, INC. Electrical Engineers | Lighting Designers 100 Howe Ave., Suite 235N

Sacramento, CA 95825-8217 www.mneilsengineering.com Tel: (916) 923-4400 Fax: (916) 923-4410 PROJECT #: 20125.21

**Sheet Scale** Sheet Number

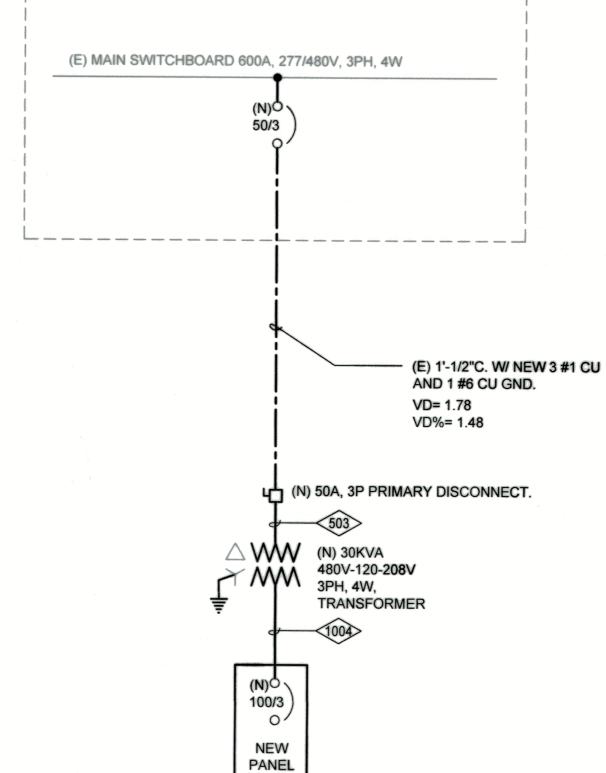
5463. 10 C





5463.126

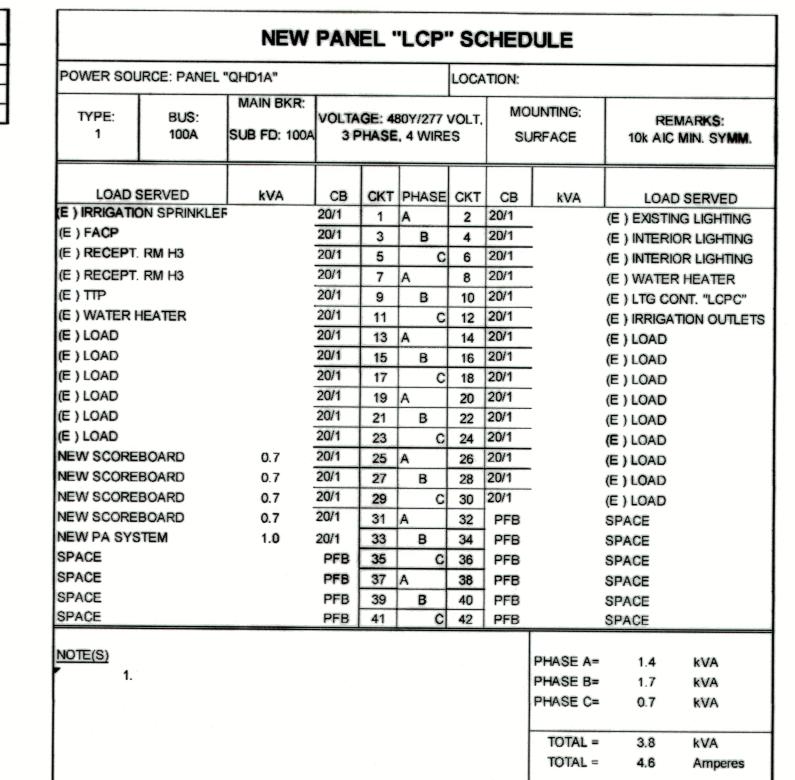
FEEDER SCHEDULE DESCRIPTION 1-1/4" C., 3 #6, AND 1 #10 GND. 1-1/4" C., 4 #2, AND 1 #8 GND.

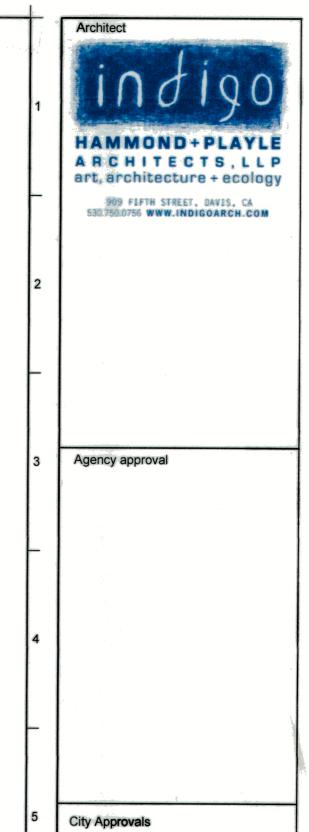


UNPAVED AREA PAVED AREA  12" TYP. AT (E) PAVING
AT (E) PAVING
CLASS 2 AB THICKNESS EQUAL TO EXISTING AND NOT LESS THAN 6" MIN. AB THICKNESS COMPACTED TO 95%.  UNPAVED AREAS, REPLACE WITH IDENTICAL MATERIAL AND FINISH. MINIMUM THICKNESS OF REPAIR TO MATCH EXISTING MATERIAL  —EXISTING ROAD SURFACE
CONDUIT TRACER 2" TO 4"  ABOVE CONDUIT (OR INSIDE ONE CONDUIT)  2" MIN.  WIN. 2" SAND PAD  WIDTH AS NEEDED  BACKFILL NATURAL SOIL. COMPACTION TO 98% OF ORIGINAL. PROVIDE REPORT TO ENGINEER OF RECORD (EOR).  QUANTITY AND SIZE OF CONDUITS PER ELECTRICAL PLAN ROCK FREE SAND MINIMUM 2-3/4" GRAVEL PAD FOR DRAINAGE

- 1. 12" MIN. RADIAL CONDUIT SEPARATION FROM EXISTING UTILITIES AND CONDUITS.
- 2. IN THE CASES WHERE THERE ARE EXISTING CONDUITS AT 36" DEPTH, THE NEW CONDUITS ARE TO GO UNDER WITH THE MINIMUM SPACING INDICATED.
- 3. NEW SURFACE (ASPHALT, CONCRETE, TURF, DIRT, ETC.) TO MATCH EXISTING IN QUALITY AND COLOR (FOR DIRT RESEED). PROVIDE EXPANSION JOINTS AT CONCRETE WALK TO ALIGN WITH EXISTING.







100% CDs W/ PLAN CHECK CORRECTIONS 9-20-2021 **McNair Soccer** 

Complex Phase - 2 9820 Ronald E. McNair

Way Stockton CA 95210

ONE LINE DIAGRAM, PANEL SCHEDULE, AND **DETAILS** Project Number

CR16024 Reference North

Sheet Scale **Sheet Number** 

5463.130

ENGINEERING, INC. Electrical Engineers | Lighting Designers 100 Howe Ave., Suite 235N Sacramento, CA 95825-8217 www.mneilsengineering.com Tel: (916) 923-4400 Fax: (916) 923-4410 PROJECT #: 20125.21

PANEL "LCP" PARTIAL ONE LINE DIAGRAM